

This addendum forms a part of the Contract Documents and modifies the original Documents dated **August 23, 2023** as noted below. Acknowledge receipt of this addendum in the space provided on the Official Bid Form. Failure to do so may subject the Bidder to disgualification.

Enclosed Seismic Rehabilitation Reports

Enclosed Seismic Rehabilitation Reports – Seismic Rehabilitation Reports

QUESTIONS AND CLARIFICATIONS

- Question: Will interviews be held in person?
- Answer: Yes.
- Question: Are there people required for the Interview?
- Answer: Per Section IV. F. "Those members invited to the interview are Project or Corporate Executive dedicated to the Project, the Project Manager, the Project Superintendent, and Project Estimator as well as the key individual responsible for preconstruction services shall be in attendance." If one of these members is scheduled off, then a request for another person or absence will be considered by HMK at their sole discretion

PRE-BID MEETING SIGN IN SHEET

Please review the attached sign in sheet; if corrections are required please send them to Stephen McKay at stephen.mckay@hmkco.org.

END OF ADDENDUM 1



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JOHN TUCK SCHOOL

Seismic Evaluation Report For:

JOHN TUCK ELEMENTARY SCHOOL

209 NW 10th St, Redmond, OR 97756 Redmond School District

Prepared By: ZCS Engineering & Architecture Matthew R. Smith, PE, SE, Principal 524 Main Street, Suite 2, Oregon City, OR 97045 T: 503.659.2205 | E: MattS@zcsea.com



209

OHN TUCK ELEMENTARY



Project Su	Project Summary Information					
Building Part	Building Part Name	Included in Retrofit	Year Built	Building Type***	Nonstructural Retrofits Included in Scope Y/N***	Previous Seismic Retrofit Y/N*** (Year if Yes)
А	Classroom	Ν	1947			
В	Gym/Classroom	Υ	1947	URM		
С	Classroom	Ν	1953			
D	Classroom	Ν	1964			
E	Library	Ν	1990s			
Seismic fra		isting buildin	gs with p	previous seis	smic retrofits MUST	of work and budget. I be adjusted to
Total Retro	ofit Cost	\$2,499,440				
Retrofit Square Feet 13,500						
Retrofit Cost per \$185.14 / SF Square Foot						
-	ous within a tsuna n potential or othe					No

Engineer	ing Report Checklist	
	Engineering Report Cover Page	
\boxtimes	Project Summary Page	Page 1
\boxtimes	Building Parts Identification	Page 4
\boxtimes	Statement of the Performance Objective	Page 6
	Summary of Deficiencies	
\boxtimes	Structural Seismic Deficiencies	Page 10
\boxtimes	Nonstructural Seismic Deficiencies	Page 11
	Summary of Mitigation/Retrofit	
\boxtimes	Structural Mitigation/Retrofit	Page 10
\boxtimes	Nonstructural Mitigation/Retrofit	Page 11
	Summary Construction Cost Estimate	
\boxtimes	Direct Cost	Page 14
\boxtimes	Indirect Soft Cost	Page 14
\boxtimes	Certification Statement by Engineer	Page 15
	ASCE 41-17 Tier 1 Checklist	
\boxtimes	Basic Configuration Checklist	Appendix B
\boxtimes	Building System Structural Checklist	Appendix B
\boxtimes	Nonstructural Checklist	Appendix B
\boxtimes	Retrofit Drawings & Sketches	Appendix C
\boxtimes	DOGAMI or Geotechnical Report	Appendix D
\boxtimes	Itemized Construction Cost Estimate	Appendix E
\boxtimes	Rapid Visual Screening	Appendix F

1.0 Project Introduction

Redmond School District is located in Redmond, Oregon in Deschutes County. The District operates ten schools located within the community including the property of interest, John Tuck Elementary School. The District has retained ZCS Engineering and Architecture (ZCS) to perform a seismic evaluation of John Tuck Elementary School that provides the District with an objective, comprehensive analysis of the condition of the building's seismic resisting systems. The purpose of the evaluation is to determine the seismic lateral resisting system deficiencies when compared to buildings designed using modern building codes. This evaluation was performed in accordance with the American Society of Civil Engineers "Seismic Rehabilitation of Existing Buildings ASCE/SEI 41-17".

SEISMIC EVALUATION SNAPSHOT			
Street Address	209 NW Palmer Street, Redmond, OR 97756		
Evaluation Standard	ASCE 41-17 (Tier 1 Analysis)		
Target Building Performance Level	Life Safety – BSE-2E; Immediate Occupancy – BSE-1E		
Target Non-Structural Performance Level	Hazard Reduced – BSE-2E; Position Retention – BSE-1E		
ASCE 41 Building Type	URM		
Site Soil Classification	D		
Seismic Zone Hazard Level	High		
Cost Estimate	\$2,451,125		

2.0 Building Description

The John Tuck Elementary School gymnasium was constructed in 1947 including and consists of CMU bearing walls with exterior brick veneer. The roof structure consists of straight sheathing over wood joists supported by heavy timber bowstring trusses. The walls consist of under-reinforced CMU walls bearing over continuous reinforced concrete footings and slab-on-grade foundation. The gymnasium has an approximate floor area of 13,500-square-feet. Photographs of the building parts included in this report are located in Appendix A.

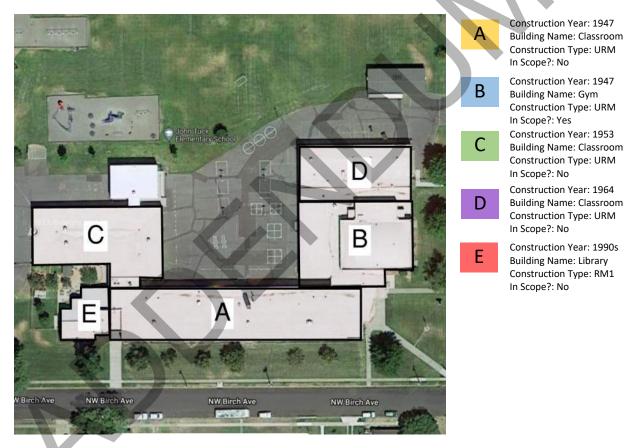


Figure 1 John Tuck Elementary Key Plan

3.0 Definition of Building Types

After reviewing the facility and the existing drawings we have determined the lateral system is defined as URM. Per ASCE 41-17 the subject structure's lateral system is defined as:

This building was initially reviewed as an RM1 construction type due to the presence of some reinforcing present in the wall construction. Through the RM1 Tier 1 evaluation it was determined that the walls are under reinforced. Accordingly, this building is classified as a URM. These buildings have a perimeter bearing walls that consist of unreinforced clay brick, stone, or concrete masonry. Interior bearing walls, where present, also consist of unreinforced clay brick, stone, or concrete masonry. In older construction, floor and roof framing consists of straight or diagonal lumber sheathing supported by wood joists, which, in turn, are supported on posts and timbers. In more recent construction, floors consist of structural panel or plywood sheathing rather than lumber sheathing. The diaphragms are flexible relative to the walls. Where they exist, ties between the walls and the diaphragms consist of anchors or bent steel plates embedded in the mortar joints and attached to framing. The foundation system may consist of a variety of elements.

4.0 Seismic Evaluation Methodology

The subject structure was evaluated using information gathered from site observations, available historic construction documents, and interviews with District staff. This information was then utilized to perform a structural evaluation as outlined in the American Society of Civil Engineer's "Seismic Evaluation and Retrofit of Existing Buildings – ASCE 41-17" (ASCE 41-17). ASCE 41-17 is referenced as the standard for seismic evaluations of existing buildings by the International Existing Building Code (IEBC) which is referenced by the Oregon Structural Specialty Code (OSSC). Further, ASCE 41-17 is the evaluation tool required by the Seismic Rehabilitation Grant Program for grant applications.

ASCE 41-17 provides several levels of evaluation (Tiers 1-3) depending on the level of evaluation and/or retrofit being performed. The Tier 1 evaluation is a quick checklist selected based on the type of construction and the performance objective of the building and is the baseline tool for preliminary seismic evaluations. In the case of this evaluation, a Tier 1 was performed to identify the likely structural deficiencies requiring retrofit to meet the performance objective stated below.

The OSSC classifies buildings into risk categories based on the type of building and occupancy type. The building's risk category informs the required performance objective post retrofit. Risk categories I and II cover low risk structures. Risk category III includes school buildings that are not required to be used as emergency shelters and are relatively low occupancy. Risk category IV includes emergency service buildings and school buildings that are required to be designed as emergency shelters (high occupancy spaces). Figure 2, below, identifies the performance objective for each risk category.

The primary objective of the adjusting performance objectives relative to risk category is to ensure that the subject building is capable of performing in the necessary manner following a seismic event. In the case of a risk category III building, the intention is to ensure that the building is adequately stable following an earthquake to provide egress for occupants out of the building. Prior to reoccupation, the building would need evaluated and significant structural damage preventing reoccupation may be present. For risk category IV structures, the intent is that the building can be inspected then immediately reoccupied following a seismic event to function in its intended role as an emergency service building or as a high occupancy space capable of acting as an emergency structure.

In accordance with the table below, section B this building is categorized as a risk category IV structure(s) and was evaluated to meet the Life Safety structural performance and Hazards Reduced nonstructural performance level for BSE-2E loading and the Immediate Occupancy structural performance and Position Retention nonstructural performance level for BSE-1E loading.

Table 2-2. Scope of Assessment Required for Tier 1 and Tier 2 with the Basic Performance Objective for Existing **Buildings (BPOE)**

	Tier 1 and 2 ^a			
Risk Category	BSE-1E	BSE-2E		
I and II	Not evaluated	Collapse Prevention Structural Performance		
	Life Safety Nonstructural Performance (3-C)	Hazards Reduced Nonstructural Performance ^b (5-D)		
III	Not evaluated	Limited Safety Structural Performance ^c		
	Position Retention Nonstructural Performance (2-B)	Hazards Reduced Nonstructural Performance ^b (4-D)		
IV	Immediate Occupancy Structural Performance	Life Safety Structural Performance ^d		
	Position Retention Nonstructural Performance (1-B)	Hazards Reduced Nonstructural Performance ^b (3-D)		

^a For Tier 1 and 2 assessments of Risk Categories I–III, Structural Performance for the BSE-1E is not explicitly

evaluated. ^b Compliance with ASCE 7 provisions for new construction is

^b Compliance with ASCE 7 provisions for new construction is deemed to comply.
 ^c For Risk Category III, the Tier 1 screening checklists shall be based on the Collapse Prevention Performance Level (S-5), except that checklist statements using the Quick Check procedures of Section 4.4.3 shall be based on *M_s* factors taken as the average of the values for Life Safety and Collapse Prevention.
 ^d For Risk Category IV, the Tier 1 screening checklists shall be based on the Collapse Prevention Performance Level (S-5), except that checklist statements using the Quick Check procedures of Section 4.4.3 shall be based on *M_s* factors for Life Safety.

Figure 2

Building Performance Objectives

Source: Table 2-2, ASCE 41-17: American Society of Civil Engineers – Seismic Evaluation and Retrofit of Existing Buildings

5.0 Seismicity

Seismic design is based on site specific parameters that relate to the location of the building relative to faults and the soil that supports the building. The United States Geologic Survey has developed seismic design data that is utilized to perform the calculations specified in ASCE 41-17. The table below summarizes the factors appropriate for computing the seismic lateral loads for the design earthquake specified in ASCE 41-17.

SITE SPECIFIC SEISMICITY	N
Soil Density	Stiff
ASCE 7-16 Soil Classification	D
BSE-1E:	
S _x	, 0.159
Sx	0.110
BSE-2E:	
S _x	, 0.40
S _{x1}	0.303
Soil Condition Amplification Factors (Fv, FA)	$F_v = 2.4 - F_a = 1.6$
ASCE 41 Site Seismicity	High

Source: SEAOC and OSHPD Seismic Design Maps, https://seismicmaps.org/

6.0 Site Specific Hazards

Site specific hazards were assessed as part of our engineering evaluation. The hazards evaluated in our analysis included liquefaction, slope failure, surface fault rupture, and tsunami potential. These potential hazards were evaluated using ASCE 41-17 guidelines, as well as information provided by the online Oregon HazVu: Statewide Geohazards Viewer, maintained by the Department of Geology and Mineral Industries (DOGAMI). Tsunami risk was evaluated using the ASCE Tsunami Hazard Tool. Results from the HazVu analysis are included in Appendix D. Unless noted below, the hazards listed above are not present at the site.

7.0 Deficiencies and Repairs

The table below summarizes both the structural and nonstructural deficiencies noted in the Tier 1 evaluation and states both the proposed retrofit methodology and the plan key note that corresponds to the scope items in the preliminary plans and the cost estimate. See Appendix B for complete Tier 1 check sheets. Drawings illustrating the proposed retrofit measures are attached in Appendix C.

Tier 1 Deficiency	Deficiency Statement	Repair Statement	Plan Key Note
Description LOAD PATH	The structure does not contain a complete, well-defined load path, including structural elements and connections, that serves to transfer the inertial forces associated with the mass of all elements of the building to the foundation.	Provide a complete, well- defined load path by installing new elements and connections as needed to transfer inertial forces from all elements of the building to the foundation.	S1
ADJACENT BUILDINGS	The clear distance between the building being evaluated and any adjacent building is less than 0.5% of the height of the shorter building in low seismicity, 1.0% in moderate seismicity, and 3.0% in high seismicity.	Provide seismic isolation joint to avoid pounding of the taller structure into the lower structure. Provide all new gravity framing and lateral resisting elements as necessary to provide building separation.	S2
SHEAR STRESS CHECK	The shear stress in the unreinforced masonry shear walls, calculated using the Quick Check procedure of Section 4.4.3.3, is greater than 30lb/in.2 for clay units and 70lb/in.2 for concrete units.	Provide new vertical lateral resisting elements.	53
WALL ANCHORAGE	Exterior concrete or masonry walls that are dependent on the diaphragm for lateral support are not anchored for out-of-plane forces at each diaphragm level with steel anchors, reinforcing dowels, or straps that are developed into the diaphragm. Connections do not have strength to resist the connection force calculated in the Quick Check procedure of Section 4.4.3.7.	Install new out-of-plane anchorage.	S4
WOOD LEDGERS	The connection between the wall panels and the diaphragm induces cross-grain bending or tension in the wood ledgers.	Install new out-of-plane anchorage.	S5

TRANSFER TO	Diaphragms are not connected for	Install new hardware for	
SHEAR WALLS	transfer of seismic forces to the shear walls, or the connections are not able to develop the shear strength of the walls or diaphragms.	transfer of seismic forces from diaphragm to shear walls.	
PROPORTIONS	The height-to-thickness ratio of the shear walls at each story is greater than the following: Top story of multi-story building 9 First story of multi-story building 15 All other conditions 13	Install new wood framed shear walls with stitch ties to support existing masonry walls for out of plane forces	S6
CROSS TIES	There are not continuous cross ties between diaphragm chords.	Provide new continuous cross ties between diaphragm chords.	58
STRAIGHT SHEATHING	Not all straight-sheathed diaphragms have aspect ratios less than 1-to-1 in the direction being considered.	Install new plywood diaphragm sheathing.	S9
SPANS	Not all wood diaphragms with spans greater than 12 ft consist of wood structural panels or diagonal sheathing.	Install new plywood diaphragm sheathing.	S10
STIFFNESS OF WALL ANCHORS	Anchors of concrete or masonry walls to wood structural elements are not installed taut or are not stiff enough to limit the relative movement between the wall and the diaphragm to no greater than 1/8 in. (3 mm) before engagement of the anchors.	Install new out-of-plane anchorage.	S11
BEAM, GIRDER, AND TRUSS SUPPORTS	Beams, girders, and trusses supported by unreinforced masonry walls or pilasters do not have independent secondary columns for support of vertical loads.	Install new secondary support for vertical load carrying framing elements.	S12
HAZARDOUS MATERIAL DISTRIBUTION	Piping or ductwork conveying hazardous materials is not braced or otherwise protected from damage that would allow hazardous material release.	Brace piping or ductwork conveying hazardous materials.	N1
FLEXIBLE COUPLINGS	Hazardous material ductwork and piping, including natural gas piping, do not have flexible couplings.	Install flexible couplings for ductwork and piping containing hazardous material, including natural gas piping.	N2
UNREINFORCED MASONRY	Unreinforced masonry or hollow-clay tile partitions are not braced at a spacing of at most 10 ft in Low or Moderate Seismicity, or at most 6 ft in	Brace unreinforced masonry or hollow-clay tile partitions.	
	High Seismicity.		N3

Redmond School District

John Tuck Elementary School Seismic Evaluation

HEAVY PARTITIONS SUPPORTED BY CEILINGS	The tops of masonry or hollow-clay tile partitions are laterally supported by an integrated ceiling system.	Independently brace the tops of masonry or hollow-clay tile partitions.	N4
TIES	Masonry veneer is not connected to the backup with corrosion-resistant ties. There is not a minimum of one tie for every 2-2/3 ft.2, or the ties have spacing greater than the following: for Life Safety in Low or Moderate Seismicity, 36 in.; for Life Safety in High Seismicity and for Position Retention in any seismicity, 24 in.	Secure existing masonry veneer with new stitch ties or remove and replace with new tied masonry veneer or other cladding system.	N5
WEAKENED PLANES	Masonry veneer is not anchored to the backup adjacent to weakened planes, such as at the locations of flashing.	Install wood framed walls with stitch ties to support existing masonry walls for out-of- plane forces.	N6
UNREINFORCED MASONRY BACKUP	Masonry backup is unreinforced.	Brace existing backup wall with new adjacent wall framing.	N7
ANCHORAGE	For veneer with concrete block or masonry backup, the backup is not positively anchored to the structure at a horizontal spacing equal to or less than 4 ft along the floors and roof.	Install wood framed walls with stitch ties to support existing masonry walls for out-of- plane forces.	N8
URM PARAPETS OR CORNICES	Laterally unsupported unreinforced masonry parapets or cornices have height-to-thickness ratios greater than the following: for Life Safety in Low or Moderate Seismicity, 2.5; for Life Safety in High Seismicity and for Position Retention in any seismicity, 1.5.	Provide bracing of parapets or cornices.	NO
APPENDAGES	Cornices, parapets, signs, and other ornamentation or appendages that extend above the highest point of anchorage to the structure or cantilever from components are not reinforced or anchored to the structural system at a spacing equal to or less than 6ft.	Provide anchorage of appendages to the structure.	N9 N10
TALL NARROW CONTENTS	Contents more than 6 ft high with a height-to-depth or height-to-width ratio greater than 3-to-1 are not anchored to the structure or to each other.	Anchor contents to the structure.	N11
FLEXIBLE COUPLINGS	Fluid and gas piping does not have flexible couplings.	Install flexible couplings for fluid and gas piping.	N12
FLUID AND GAS PIPING	Fluid and gas piping is not anchored or braced to the structure to limit spills or leaks.	Anchor and brace fluid and gas piping to the structure.	N13

In addition to the structural and nonstructural deficiencies noted above, the gravity load resisting system was reviewed to identify obvious insufficient gravity components. Insufficient gravity elements can cause failure during seismic events. These gravity deficiencies are based on visual observations of the existing structural elements. No formal structural analysis was performed during this evaluation of the gravity resisting element.

Bowstring trusses are markedly under-designed, exhibiting on-going symptoms of structural distress and can no longer be relied upon to support code prescribed gravity loading. The trusses will be retrofit and strengthened to support code required gravity loading. This is deficiency/repair/plan note S13.

Based upon ZCS's previous experience and discussions with site personnel the buildings contain hazardous materials. These materials will need to be dealt with on a case-by-case basis as they are encountered during the project.

8.0 Preliminary Construction Cost Estimate

The attached engineer's opinion of probable cost has been developed by ZCS. ZCS has a successful record of completing seismic rehabilitation projects within the State of Oregon. The prices provided in the attached cost estimate have been developed using the extensive list of past projects as a baseline for this project. These prices are based on Oregon BOLI wage rates. The cost estimate is broken down into multiple line items associated with each major task (general conditions, foundation, structural steel, MEP, etc) associated with the rehabilitation. Additional line items are included for design associated permit costs, and owner construction management. A complete breakdown of the cost estimate can be found in Appendix E.

Special Notes

- It should be noted that the cost per square-foot of the this retrofit may seem abnormally high. The higher-than-average costs are a result of the following:
 - The building is an unreinforced masonry wall structure. The walls exceed prescribed limitations and an entirely new lateral system to support seismic loading needs to be installed.

DIRECT COST				
Construction	\$1,940,400			
Engineering	\$282,500			
Construction Management	\$61,700			
Relocation	\$28,000			
Construction Contingency	\$186,840			
TOTALS AND SUMMARY				
Total Cost Estimate	\$2,499,440			
Match Funds	\$0			
Total Amount Requested from SRGP	\$2,499,440			
Total Area	13,500			
Cost/Square Foot	\$185.14			

9.0 Conclusion and Certification Statement

The findings described in this report have been limited to the lateral force-resisting structural system and general assessment of the gravity force-resisting elements. Based on our visual observations, we find the structure to be in relatively good condition and generally safe for occupancy. No significant damage to the existing structural system was discovered.

Given the current condition of the structure, the current code section on existing buildings does not mandate that upgrades are required unless the building is scheduled for repairs, alterations, additions, or change in occupancy. To clarify, upgrades outlined in this report are strictly at the discretion of the District.

Please contact our office if you would like to discuss our findings. Please review the attached schematic drawings that can be used to refine a scope and budget.

Certification Statement

ZCS Engineering & Architecture's professional staff has reviewed the subject building and the deficiencies noted in the Tier 1 evaluation, developed seismic retrofit solutions to rectify the deficiencies, and developed the engineering cost estimate. The project cost estimate was developed by ZCS based on unit costs from our extensive list of past seismic retrofit projects as a baseline. We certify to the best of our knowledge, based on known and readily identifiable existing conditions, that all the seismic deficiencies present in the building are included in the retrofit scope of work and that all the retrofit's scope of work elements are included in the cost estimate.

Matthew R. Smith, PE, SE

Redmond School District John Tuck Elementary School Seismic Evaluation

Appendix A: Figures

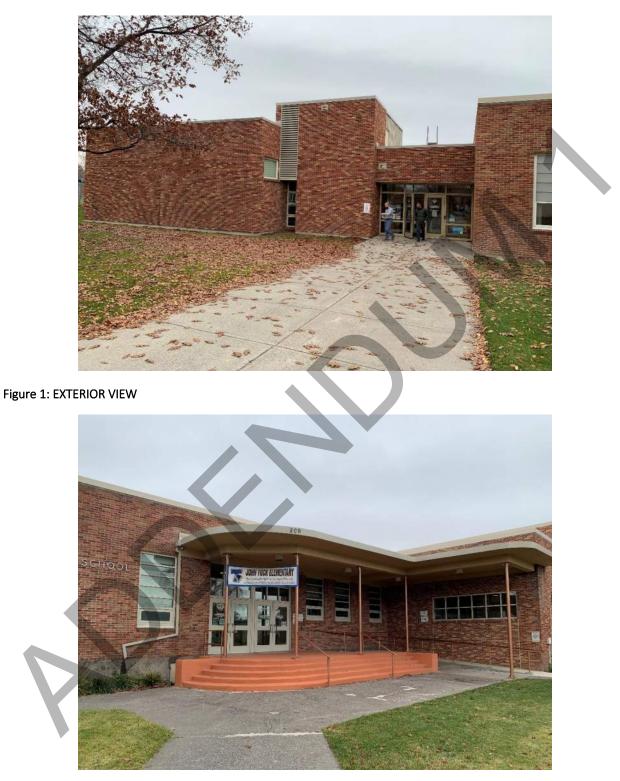


Figure 2: ENTRANCE



Figure 3: GYM ENTRANCE



Figure 4: GYM EXTERIOR





Figure 5: CLASSROOM EXTERIOR

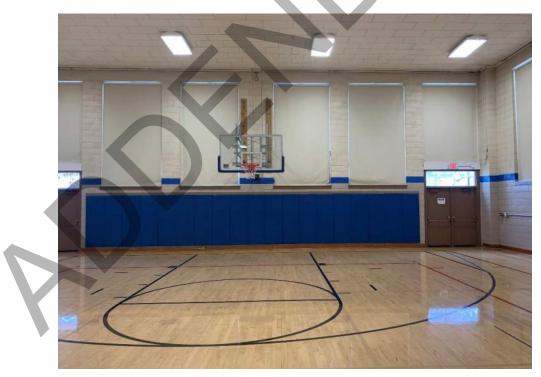


Figure 6: GYM INTERIOR

Redmond School District John Tuck Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix B: Tier 1 Check Sheets

ASCE 41-17 Tier 1 Checklists

FIRM:	
PROJECT NAME:	
SEISMICITY LEVEL:	
PROJECT NUMBER:	
COMPLETED BY:	
DATE COMPLETED:	
REVIEWED BY:	
REVIEW DATE:	

Project Name
Project Number

17.1.210 Basic Configuration Checklist

Table 17-3. Immediate Occupancy Basic Configuration Checklist

					Tier 2	Commentary
Status	5			Evaluation Statement	Reference	Reference Comments
Very L	.ow Seis	micity				
Buildi	ng Syste	m—Gene	eral			
С	NC	N/A	U	LOAD PATH: The structure	5.4.1.1	A.2.1.1
				contains a complete, well-defined		
				load path, including structural		
				elements and connections, that		
				serves to transfer the inertial forces		
				associated with the mass of all		
				elements of the building to the		
				foundation.		
С	NC	N/A	U	ADJACENT BUILDINGS: The clear	5.4.1.2	A.2.1.2
				distance between the building		
				being evaluated and any adjacent		
				building is greater than 0.5% of		
				the height of the shorter building		
				in low seismicity, 1.0% in moderate		
				seismicity, and 3.0% in high		
				seismicity.		
С	NC	N/A	U	MEZZANINES: Interior mezzanine	5.4.1.3	A.2.1.3
				levels are braced independently		
				from the main structure or are		
				anchored to the seismic-force-		
				resisting elements of the main		
				structure.		
Buildiı	ng Syste	m—Buila	ling Co	nfiguration		
С	NC	N/A	U	WEAK STORY: The sum of the shear	5.4.2.1	A.2.2.2
				strengths of the seismic-force-		
				resisting system in any story in		
				each direction is not less than 80%		
				of the strength in the adjacent		
			,	story above.		
С	NC	N/A	U	SOFT STORY: The stiffness of the	5.4.2.2	A.2.2.3
				seismic-force-resisting system in		
				any story is not less than 70% of		
				the seismic-force-resisting system		
				stiffness in an adjacent story above		
				or less than 80% of the average		
				seismic-force-resisting system		
				stiffness of the three stories above.		
С	NC	N/A	U	VERTICAL IRREGULARITIES: All	5.4.2.3	A.2.2.4
				vertical elements in the seismic-		
				force-resisting system are		
				continuous to the foundation.		

						Project Name Project Numb	
						riojectivanic	
					<u> </u>	4.2.2.5	
C	NC	N/A	U	GEOMETRY: There are no changes in the net horizontal dimension of	5.4.2.4	A.2.2.5	
				the seismic-force-resisting system			
				of more than 30% in a story			
				relative to adjacent stories,			
				excluding one-story penthouses and mezzanines.			
	NC	N/A	U	MASS: There is no change in	5.4.2.5	A.2.2.6	
-			-	effective mass of more than 50%			
				from one story to the next. Light			
				roofs, penthouses, and			
				mezzanines need not be considered.			
С	NC	N/A	U	TORSION: The estimated distance	5.4.2.6	A.2.2.7	
				between the story center of mass			
				and the story center of rigidity is			
				less than 20% of the building			
				width in either plan dimension.			*
					Tier 2	Commentary	
Status	5			Evaluation Statement	Tier 2 Reference	Commentary Reference	Comments
		y (Comp	lete the	Evaluation Statement e Following Items in Addition to the	Reference	Reference	
Low S	eismicit	y (Comp Hazards	lete the		Reference	Reference	
Low S	eismicit		lete the	e Following Items in Addition to the LIQUEFACTION: Liquefaction-	Reference	Reference	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I NC	Hazards N/A	U	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S Geolo	eismicit gic Site I NC	N/A	U 	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S Geolo	eismicit gic Site I NC	N/A	U 	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S Geolo	eismicit gic Site I NC	N/A	U 	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S Geolo	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure.	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	
Low S Geolo	eismicit gic Site I NC	N/A	U 	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S Geolo C C	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface fault rupture and surface	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	
Low S Geolo C C	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	

Project Name	
Project Number	

Status	:			Evaluation Statement	Tier 2 Reference	Commentary Reference	Comments
		High Sei	smicity	y (Complete the Following Items in .			
		nfigurati					
c		N/A	U	OVERTURNING: The ratio of the least horizontal dimension of the seismic-force-resisting system at the foundation level to the building height (base/height) is greater than 0.6 <i>S</i> _a .	5.4.3.3	A.6.2.1	N
c		N/A		TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C.	5.4.3.4	A.6.2.2	

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Project Name	
Project Number	

17.18IO Structural Checklist for Building Types URM: Unreinforced Masonry Bearing Walls with Flexible Diaphragms and URMa: Unreinforced Masonry Bearing Walls with Stiff Diaphragms

				Tier 2	Commentary	
Status			Evaluation Statement	Reference	Reference	Comments
Very Low S	Seismici	ty				
Seismic-Fo	rce-Resi	sting S	System			
C NC	N/A	U	REDUNDANCY: The number of lines of	5.5.1.1	A.3.2.1.1	
			shear walls in each principal direction			
			is greater than or equal to 2.			
C NC	N/A	U	SHEAR STRESS CHECK: The shear	5.5.3.1.1	A.3.2.5.1	
			stress in the unreinforced masonry			
			shear walls, calculated using the Quick			
			Check procedure of Section 4.4.3.3, is			
			less than 30 lb/in. ² (0.21 MPa) for clay			
			units and 70 lb/in. ² (0.48 MPa) for			
			concrete units.			
Connection						
C NC	N/A	U	WALL ANCHORAGE: Exterior concrete	5.7.1.1	A.5.1.1	
			or masonry walls that are dependent			
			on the diaphragm for lateral support			
			are anchored for out-of-plane forces			
			at each diaphragm level with steel			
			anchors, reinforcing dowels, or straps			
			that are developed into the			
			diaphragm. Connections have			
			strength to resist the connection force calculated in the Quick Check			
			procedure of Section 4.4.3.7.			
C NC	N/A	U	WOOD LEDGERS: The connection	5.7.1.3	A.5.1.2	
		Č	between the wall panels and the	5.7.1.5	A.J.1.2	
			diaphragm does not induce cross-			
			grain bending or tension in the wood			
			ledgers.			
C NC	N/A	U	TRANSFER TO SHEAR WALLS:	5.7.2	A.5.2.1	
	,.		Diaphragms are connected for			
			transfer of seismic forces to the shear			
			walls, and the connections are able to			
		~	develop the lesser of the shear			
			strength of the walls or diaphragms.			
C NC	N/A	U	GIRDER-COLUMN CONNECTION:	5.7.4.1	A.5.4.1	
		_	There is a positive connection using			
			plates, connection hardware, or straps			
			between the girder and the column			
			between the grader and the column			

Table 17-37. Immediate Occupancy Structural Checklist for Building Types URM and URMa

						Project Nar	ne
						Project Nur	mber
Foun	dation	n Systen	n				
С	NC	N/A	U	DEEP FOUNDATIONS: Piles and piers		A.6.2.3	
				are capable of transferring the lateral			
				forces between the structure and the			
				soil.			
С	NC	N/A	U	SLOPING SITES: The difference in		A.6.2.4	
				foundation embedment depth from			
				one side of the building to another			
				does not exceed one story high.			
					Tier 2	Commentary	
Statu	IS			Evaluation Statement	Reference	Reference	Comments
Low,	Mode	rate, ar	nd Hig	h Seismicity (Complete the Following I	tems in Additi	on to the Items f	for Very Low Seismicity)
Seisn	nic-For	rce-Resi	isting :	System			
С	NC	N/A	U	PROPORTIONS: The height-to-	5.5.3.1.2	A.3.2.5.2	
				thickness ratio of the shear walls at			
				each story is less than the following:			
				Top story of multi-story building 9			Ψ.
				First story of multi-story building 15			
				All other conditions 13			
С	NC	N/A	U	MASONRY LAYUP: Filled collar joints of	5.5.3.4.1	A.3.2.5.3	
				multi-wythe masonry walls have			
				negligible voids.			
	-	s (Stiff					
С	NC	N/A	U	OPENINGS AT SHEAR WALLS:	5.6.1.3	A.4.1.4	
	\square			Diaphragm openings immediately			
				adjacent to the shear walls are less			
				than 15% of the wall length.			
С	NC	N/A	U	OPENINGS AT EXTERIOR MASONRY	5.6.1.3	A.4.1.6	
				SHEAR WALLS: Diaphragm openings			
				immediately adjacent to exterior			
				masonry shear walls are not greater than 4 ft (1.2 m) long.			
c	NC	N/A	U	PLAN IRREGULARITIES: There is tensile	5.6.1.4	A.4.1.7	
				capacity to develop the strength of	5.0.1.4	A.4.1.7	
				the diaphragm at reentrant corners or			
				other locations of plan irregularities.			
С	NC	N/A	U	DIAPHRAGM REINFORCEMENT AT	5.6.1.5	A.4.1.8	
				OPENINGS: There is reinforcing around	5.0.1.5	/	
				all diaphragm openings larger than			
				50% of the building width in either			
				major plan dimension.			
Flexi	ble Dia	phrag	ns	· · · · · · · · · · · · · · · · · · ·			
C	NC	N/A	U	CROSS TIES: There are continuous	5.6.1.2	A.4.1.2	
-	-		-	cross ties between diaphragm chords.			
				, 5			

						Project Name Project Number
					562	A 4 2 1
c	NC	N/A	U	STRAIGHT SHEATHING: All straight- sheathed diaphragms have aspect	5.6.2	A.4.2.1
				ratios less than 1-to-1 in the direction		
				being considered.		
C	NC	N/A	U	SPANS: All wood diaphragms with	5.6.2	A.4.2.2
	\square			spans greater than 12 ft (3.6 m) consist		
				of wood structural panels or diagonal		
c	NC	N/A	U	sheathing. DIAGONALLY SHEATHED AND	5.6.2	A.4.2.3
				UNBLOCKED DIAPHRAGMS: All	5.0.2	1.1.2.5
				diagonally sheathed or unblocked		
				wood structural panel diaphragms		
				have horizontal spans less than 30 ft		
				(9.2 m) and aspect ratios less than or equal to 3-to-1.		
с	NC	N/A	U	NONCONCRETE FILLED DIAPHRAGMS:	5.6.3	A.4,3.1
-				Untopped metal deck diaphragms or	51015	
				metal deck diaphragms with fill other		
				than concrete consist of horizontal		
				spans of less than 40 ft (12.2 m) and		
				have aspect ratios less than 4-to-1.		A 471
c	NC	N/A	U	OTHER DIAPHRAGMS: Diaphragms do not consist of a system other than	5.6.5	A.4.7.1
				wood, metal deck, concrete, or		
				horizontal bracing.		
Conn	ection	s				
С	NC	N/A	U	STIFFNESS OF WALL ANCHORS:	5.7.1.2	A.5.1.4
				Anchors of concrete or masonry walls		
				to wood structural elements are installed taut and are stiff enough to		
				limit the relative movement between		
				the wall and the diaphragm to no		
				greater than 1/8 in. (3 mm) before		
				engagement of the anchors.		
С	NC	N/A	U	BEAM, GIRDER, AND TRUSS SUPPORTS:	5.7.4.4	A.5.4.5
				Beams, girders, and trusses supported		
				by unreinforced masonry walls or pilasters have independent secondary		
				columns for support of vertical loads.		

Project Name
Project Number

17.19 Nonstructural Checklist

Table 17-38. Nonstructural Checklist

Status			Evaluation Statement ^{a,b}	Tier 2 Reference	Commentary Reference	Comments
Life Safe	ty Systen	15				
C NO	: N/A	U	HR—not required; LS—LMH; PR—LMH. FIRE	13.7.4	A.7.13.1	
			SUPPRESSION PIPING: Fire suppression piping is			
			anchored and braced in accordance with NFPA-13.			
C NO	N/A	U	HR—not required; LS—LMH; PR—LMH. FLEXIBLE	13.7.4	A.7.13.2	
			COUPLINGS: Fire suppression piping has flexible			
			couplings in accordance with NFPA-13.		<u>^</u>	
C NO	N/A	U	HR—not required; LS—LMH; PR—LMH.	13.7.7	A.7.12.1	¥.
			EMERGENCY POWER: Equipment used to power or			
			control Life Safety systems is anchored or braced.			
C NO	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR AND	13.7.6	A.7.14.1	
			SMOKE DUCTS: Stair pressurization and smoke			
			control ducts are braced and have flexible			
			connections at seismic joints.			
C NO	N/A	U	HR—not required; LS—MH; PR—MH. SPRINKLER	13.7.4	A.7.13.3	
			CEILING CLEARANCE: Penetrations through panelized		1	
			ceilings for fire suppression devices provide			
			clearances in accordance with NFPA-13.			
C NO	. N/A	U	HR—not required; LS—not required; PR—LMH.	13.7.9	A.7.3.1	
			EMERGENCY LIGHTING: Emergency and egress			
			lighting equipment is anchored or braced.			
Hazardo	us Mater	ials				
C NO	N/A	U	HR—LMH; LS—LMH; PR—LMH. HAZARDOUS	13.7.1	A.7.12.2	
			MATERIAL EQUIPMENT: Equipment mounted on			
			vibration isolators and containing hazardous material			
			is equipped with restraints or snubbers.			
C NO	: N/A	U	HR—LMH; LS—LMH; PR—LMH. HAZARDOUS	13.8.3	A.7.15.1	
			MATERIAL STORAGE: Breakable containers that hold			
			hazardous material, including gas cylinders, are			
			restrained by latched doors, shelf lips, wires, or other			
			methods.			
C NO	: N/A	U	HR—MH; LS—MH; PR—MH. HAZARDOUS MATERIAL	13.7.3	A.7.13.4	
	רו ר		DISTRIBUTION: Piping or ductwork conveying	13.7.5		
			hazardous materials is braced or otherwise protected			
			from damage that would allow hazardous material			
			release.			
C NO	: N/A	U	HR—MH; LS—MH; PR—MH. SHUTOFF VALVES:	13.7.3	A.7.13.3	
			Piping containing hazardous material, including	13.7.5		
			natural gas, has shutoff valves or other devices to			
	Ŧ		limit spills or leaks.			
C NO	N/A	U	HR—LMH; LS—LMH; PR—LMH. FLEXIBLE	13.7.3	A.7.15.4	
			COUPLINGS: Hazardous material ductwork and	13.7.5		
			piping, including natural gas piping, have flexible			
			couplings.			

					Project Na	ime
					Project Nu	umber
c	NC	N/A	U	HR—MH; LS—MH; PR—MH. PIPING OR DUCTS	13.7.3	A.7.13.6
				CROSSING SEISMIC JOINTS: Piping or ductwork	13.7.5	
				carrying hazardous material that either crosses	13.7.6	
				seismic joints or isolation planes or is connected to		
				independent structures has couplings or other details		
				to accommodate the relative seismic displacements.		
Parti	tions					
C	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. UNREINFORCED	13.6.2	A.7.1.1
_				MASONRY: Unreinforced masonry or hollow-clay tile		
				partitions are braced at a spacing of at most 10 ft (3.0		
				m) in Low or Moderate Seismicity, or at most 6 ft (1.8		
				m) in High Seismicity.		
с	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. HEAVY PARTITIONS	13.6.2	A.7.2.1
				SUPPORTED BY CEILINGS: The tops of masonry or		
				hollow-clay tile partitions are not laterally supported		
				by an integrated ceiling system.		
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. DRIFT: Rigid	13.6.2	A.7.1.2
				cementitious partitions are detailed to accommodate		
				the following drift ratios: in steel moment frame,		
				concrete moment frame, and wood frame buildings,		
				0.02; in other buildings, 0.005.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2	A.7.2.1
				LIGHT PARTITIONS SUPPORTED BY CEILINGS: The tops		
				of gypsum board partitions are not laterally		
				supported by an integrated ceiling system.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2	A.7.1.3
				STRUCTURAL SEPARATIONS: Partitions that cross		
				structural separations have seismic or control joints.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2	A.7.1.4
			\square	TOPS: The tops of ceiling-high framed or panelized		
				partitions have lateral bracing to the structure at a		
				spacing equal to or less than 6 ft (1.8 m).		
Ceilir	ngs					
С	NC	N/A	U	HR—H; LS—MH; PR—LMH. SUSPENDED LATH AND	13.6.4	A.7.2.3
				PLASTER: Suspended lath and plaster ceilings have		
				attachments that resist seismic forces for every 12 ft ²		
				(1.1 m ²) of area.		
С	NC	N/A	U	HR—not required; LS—MH; PR—LMH. SUSPENDED	13.6.4	A.7.2.3
				GYPSUM BOARD: Suspended gypsum board ceilings		
				have attachments that resist seismic forces for every		
				12 ft ² (1.1 m ²) of area.		

Project Name	
Project Number	

C NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.2
			INTEGRATED CEILINGS: Integrated suspended ceilings		
			with continuous areas greater than 144 ft ² (13.4 m ²)		
			and ceilings of smaller areas that are not surrounded		
			by restraining partitions are laterally restrained at a		
			spacing no greater than 12 ft (3.6 m) with members		
			attached to the structure above. Each restraint		
			location has a minimum of four diagonal wires and		
			compression struts, or diagonal members capable of		
			resisting compression.		
C NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.4
			EDGE CLEARANCE: The free edges of integrated		
			suspended ceilings with continuous areas greater		
			than 144 ft ² (13.4 m ²) have clearances from the		
			enclosing wall or partition of at least the following: in		
			Moderate Seismicity, 1/2 in. (13 mm); in High		
			Seismicity, 3/4 in. (19 mm).		
C NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.5
			CONTINUITY ACROSS STRUCTURE JOINTS: The ceiling		
			system does not cross any seismic joint and is not		
			attached to multiple independent structures.		
C NC	N/A	U	HR—not required; LS—not required; PR—H. EDGE	13.6.4	A.7.2.6
			SUPPORT: The free edges of integrated suspended		
			ceilings with continuous areas greater than 144 ft ²		
			(13.4 m ²) are supported by closure angles or channels		
			not less than 2 in. (51 mm) wide.		
C NC	N/A	U	HR—not required; LS—not required; PR—H.	13.6.4	A.7.2.7
			SEISMIC JOINTS: Acoustical tile or lay-in panel ceilings		
			have seismic separation joints such that each		
			continuous portion of the ceiling is no more than		
			2,500 ft ² (232.3 m ²) and has a ratio of long-to-short		
			dimension no more than 4-to-1.		
Light Fixtur	es				
C NC	N/A	U	HR—not required; LS—MH; PR—MH.	13.6.4	A.7.3.2
		Π	INDEPENDENT SUPPORT: Light fixtures that weigh	13.7.9	
			more per square foot than the ceiling they penetrate		
			are supported independent of the grid ceiling		
			suspension system by a minimum of two wires at		
			diagonally opposite corners of each fixture.		
			7		
		-			

					Project Na	ime
					Project Nu	umber
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.9	A.7.3.3
				PENDANT SUPPORTS: Light fixtures on pendant		
				supports are attached at a spacing equal to or less		
				than 6 ft. Unbraced suspended fixtures are free to		
				allow a 360-degree range of motion at an angle not		
				less than 45 degrees from horizontal without		
				contacting adjacent components. Alternatively, if		
				rigidly supported and/or braced, they are free to		
				move with the structure to which they are attached		
				without damaging adjoining components.		
				Additionally, the connection to the structure is		
				capable of accommodating the movement without		
				failure.		
c	NC	N/A	U	HR—not required; LS—not required; PR—H. LENS	13.7.9	A.7.3.4
			Ē	COVERS: Lens covers on light fixtures are attached	15.7.5	
				with safety devices.		
Clad	ding ar	nd Glaz	ing	,		
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. CLADDING ANCHORS:	13.6.1	A.7.4.1
				Cladding components weighing more than 10 lb/ft ²		
				(0.48 kN/m ²) are mechanically anchored to the		
				structure at a spacing equal to or less than the		
				following: for Life Safety in Moderate Seismicity, 6 ft		
				(1.8 m); for Life Safety in High Seismicity and for		
				Position Retention in any seismicity, 4 ft (1.2 m)		
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. CLADDING	13.6.1	A.7.4.3
				ISOLATION: For steel or concrete moment-frame		
				buildings, panel connections are detailed to		
				accommodate a story drift ratio by the use of rods		
				attached to framing with oversize holes or slotted		
				holes of at least the following: for Life Safety in		
				Moderate Seismicity, 0.01; for Life Safety in High		
				Seismicity and for Position Retention in any		
				seismicity, 0.02, and the rods have a length-to-		
				diameter ratio of 4.0 or less.		
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. MULTI-STORY PANELS:	13.6.1	A.7.4.4
				For multi-story panels attached at more than one		
				floor level, panel connections are detailed to		
				accommodate a story drift ratio by the use of rods		
				attached to framing with oversize holes or slotted		
				holes of at least the following: for Life Safety in		
				Moderate Seismicity, 0.01; for Life Safety in High		
	`			Seismicity and for Position Retention in any		
				seismicity, 0.02, and the rods have a length-to-		
		Ŧ		diameter ratio of 4.0 or less.		

Project Name Project Number

C	NC	N/A	U	HR—not required; LS—MH; PR—MH. THREADED	13.6.1	A.7.4.9
				RODS: Threaded rods for panel connections detailed		
				to accommodate drift by bending of the rod have a		
				length-to-diameter ratio greater than 0.06 times the		
				story height in inches for Life Safety in Moderate		
				Seismicity and 0.12 times the story height in inches		
				for Life Safety in High Seismicity and Position		
				Retention in any seismicity.		
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. PANEL CONNECTIONS:	13.6.1.4	A.7.4.5
				Cladding panels are anchored out of plane with a		
				minimum number of connections for each wall panel,		
				as follows: for Life Safety in Moderate Seismicity, 2		
				connections; for Life Safety in High Seismicity and for		
				Position Retention in any seismicity, 4 connections.		
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. BEARING	13.6.1.4	A.7.4.6
				CONNECTIONS: Where bearing connections are used,		
				there is a minimum of two bearing connections for		
				each cladding panel.		
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. INSERTS: Where	13.6.1.4	A.7.4.7
				concrete cladding components use inserts, the inserts		
				have positive anchorage or are anchored to		
				reinforcing steel.		
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. OVERHEAD	13.6.1.5	A.7.4.8
				GLAZING: Glazing panes of any size in curtain walls		
				and individual interior or exterior panes more than 16		
				ft^2 (1.5 m ²) in area are laminated annealed or		
				laminated heat-strengthened glass and are detailed		
				to remain in the frame when cracked.		
Maso	onry Ve	neer				
C	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. TIES:	13.6.1.2	A.7.5.1
-			-	Masonry veneer is connected to the backup with		
				corrosion-resistant ties. There is a minimum of one tie		
				for every 2-2/3 ft ² (0.25 m ²), and the ties have spacing		
				no greater than the following: for Life Safety in Low or		
				Moderate Seismicity, 36 in. (914 mm); for Life Safety in		
				High Seismicity and for Position Retention in any		
				seismicity, 24 in. (610 mm).		
c	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. SHELF	13.6.1.2	A.7.5.2
ر س	NC	N/A		ANGLES: Masonry veneer is supported by shelf angles	15.0.1.2	A.7.3.2
				or other elements at each floor above the ground		
				-		
	NC	NI/A		floor.	12612	A 7 5 2
C	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. WEAKENED	13.6.1.2	A.7.5.3
				PLANES: Masonry veneer is anchored to the backup		
				adjacent to weakened planes, such as at the locations		
				of flashing.		

					Project Name		
					Project Number		
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. UNREINFORCED	13.6.1.1	A.7.7.2	
				MASONRY BACKUP: There is no unreinforced masonry	13.6.1.2		
				backup.			
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. STUD	13.6.1.1	A.7.6.1	
				TRACKS: For veneer with cold-formed steel stud	13.6.1.2		
				backup, stud tracks are fastened to the structure at a			
				spacing equal to or less than 24 in. (610 mm) on			
				center.			
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. ANCHORAGE:	13.6.1.1	A.7.7.1	
				For veneer with concrete block or masonry backup,	13.6.1.2		
				the backup is positively anchored to the structure at a			
				horizontal spacing equal to or less than 4 ft along the			
				floors and roof.			
C	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.1.2	A.7.5.6	
				WEEP HOLES: In veneer anchored to stud walls, the			
				veneer has functioning weep holes and base flashing.			
C	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.1.1	A.7.6.2	
				OPENINGS: For veneer with cold-formed-steel stud	13.6.1.2		
				backup, steel studs frame window and door			
0			0	openings.			
	NC	N/A		mentation, and Appendages HR—LMH; LS—LMH; PR—LMH. URM PARAPETS OR	13.6.5	A.7.8.1	
с 		IN/A	U	CORNICES: Laterally unsupported unreinforced	15.0.5	A:7.8:1	
				masonry parapets or cornices have height-to-			
				thickness ratios no greater than the following: for Life			
				Safety in Low or Moderate Seismicity, 2.5; for Life			
				Safety in High Seismicity and for Position Retention in			
				any seismicity, 1.5.			
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. CANOPIES:	13.6.6	A.7.8.2	
				Canopies at building exits are anchored to the			
				structure at a spacing no greater than the following:			
				for Life Safety in Low or Moderate Seismicity, 10 ft (3.0			
				m); for Life Safety in High Seismicity and for Position			
				Retention in any seismicity, 6 ft (1.8 m).			
С	NC	N/A	U	HR—H; LS—MH; PR—LMH. CONCRETE PARAPETS:	13.6.5	A.7.8.3	
				Concrete parapets with height-to-thickness ratios			
				greater than 2.5 have vertical reinforcement.			
С	NC	N/A	U	HR—MH; LS—MH; PR—LMH. APPENDAGES:	13.6.6	A.7.8.4	
				Cornices, parapets, signs, and other ornamentation or			
				appendages that extend above the highest point of			
				anchorage to the structure or cantilever from			
				components are reinforced and anchored to the			
				structural system at a spacing equal to or less than 6			
				ft (1.8 m). This evaluation statement item does not			
				apply to parapets or cornices covered by other			
				evaluation statements.			

Project Name Project Name
Project Number

Maso	onry Ch	imneys	5			
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. URM CHIMNEYS:	13.6.7	A.7.9.1
				Unreinforced masonry chimneys extend above the		
				roof surface no more than the following: for Life		
				Safety in Low or Moderate Seismicity, 3 times the		
				least dimension of the chimney; for Life Safety in High		
				Seismicity and for Position Retention in any		
				seismicity, 2 times the least dimension of the		
				chimney.		
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. ANCHORAGE:	13.6.7	A.7.9.2
				Masonry chimneys are anchored at each floor level, at		
				the topmost ceiling level, and at the roof.		
Stair	s					
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR	13.6.2	A.7.10.1
				ENCLOSURES: Hollow-clay tile or unreinforced	13.6.8	
				masonry walls around stair enclosures are restrained		
				out of plane and have height-to-thickness ratios not		
				greater than the following: for Life Safety in Low or		
				Moderate Seismicity, 15-to-1; for Life Safety in High		
				Seismicity and for Position Retention in any		
				seismicity, 12-to-1.		
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR	13.6.8	A.7.10.2
_		-		DETAILS: The connection between the stairs and the		
				structure does not rely on post-installed anchors in		
				concrete or masonry, and the stair details are capable		
				of accommodating the drift calculated using the		
				Quick Check procedure of Section 4.4.3.1 for		
				moment-frame structures or 0.5 in. for all other		
				structures without including any lateral stiffness		
				contribution from the stairs.		
Cont	ents ar	nd Furn	ishina			
<u> </u>	NC	N/A	U	HR—LMH; LS—MH; PR—MH. INDUSTRIAL STORAGE	13.8.1	A.7.11.1
			Č	RACKS: Industrial storage racks or pallet racks more	15.0.1	//
				than 12 ft high meet the requirements of ANSI/RMI		
				MH 16.1 as modified by ASCE 7, Chapter 15.		
c	NC	N/A	U	HR—not required; LS—H; PR—MH. TALL NARROW	13.8.2	A.7.11.2
			U	CONTENTS: Contents more than 6 ft (1.8 m) high with	13.0.2	A.7.11.2
	Ц			a height-to-depth or height-to-width ratio greater		
				than 3-to-1 are anchored to the structure or to each		
				other.		
c	NC	N/A	U	HR—not required; LS—H; PR—H. FALL-PRONE	13.8.2	A.7.11.3
<u> </u>			_	CONTENTS: Equipment, stored items, or other	10.0.2	
				contents weighing more than 20 lb (9.1 kg) whose		
				center of mass is more than 4 ft (1.2 m) above the		
				adjacent floor level are braced or otherwise		
				restrained.		
				iestianieu.		

Legend: C = Compliant, NC = Noncompliant, N/A = Not Applicable, U = Unknown

					Project I	Name	
					Project l	Number	
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.10	A.7.11.4	
				ACCESS FLOORS: Access floors more than 9 in. (229			
				mm) high are braced.			
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.7.7	A.7.11.5	
				EQUIPMENT ON ACCESS FLOORS: Equipment and	13.6.10		
				other contents supported by access floor systems are			
				anchored or braced to the structure independent of			
				the access floor.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.8.2	A.7.11.6	
				SUSPENDED CONTENTS: Items suspended without			
				lateral bracing are free to swing from or move with			
				the structure from which they are suspended without			
				damaging themselves or adjoining components.			
				ll Equipment			
C	NC	N/A	U	HR—not required; LS—H; PR—H. FALL-PRONE	13.7.1	A.7.12.4	
				EQUIPMENT: Equipment weighing more than 20 lb	13.7.7		
				(9.1 kg) whose center of mass is more than 4 ft (1.2 m)			
				above the adjacent floor level, and which is not in-			
c	NC	N/A	U	line equipment, is braced. HR—not required; LS—H; PR—H. IN-LINE	13.7.1	A.7.12.5	
ر ر		IN/A	0	EQUIPMENT: Equipment installed in line with a duct	15.7.1	A.7.12.5	
				or piping system, with an operating weight more			
				than 75 lb (34.0 kg), is supported and laterally braced			
				independent of the duct or piping system.			
c	NC	N/A	U	HR—not required; LS—H; PR—MH. TALL NARROW	13.7.1	A.7.12.6	
				EQUIPMENT: Equipment more than 6 ft (1.8 m) high	13.7.7		
				with a height-to-depth or height-to-width ratio			
				greater than 3-to-1 is anchored to the floor slab or			
				adjacent structural walls.			
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.9	A.7.12.7	
				MECHANICAL DOORS: Mechanically operated doors			
				are detailed to operate at a story drift ratio of 0.01.			
C	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12.8	
				SUSPENDED EQUIPMENT: Equipment suspended	13.7.7		
				without lateral bracing is free to swing from or move			
				with the structure from which it is suspended without			
				damaging itself or adjoining components.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12.9	
				VIBRATION ISOLATORS: Equipment mounted on			
				vibration isolators is equipped with horizontal			
				restraints or snubbers and with vertical restraints to			
	NC			resist overturning.	10 7 1	A 7 10 10	
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12.10	
				HEAVY EQUIPMENT: Floor-supported or platform-	13.7.7		
				supported equipment weighing more than 400 lb			
				(181.4 kg) is anchored to the structure.			

Legend: C = Compliant, NC = Noncompliant, N/A = Not Applicable, U = Unknown

					Project Na		
					Project N		
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.7	A.7.12.11	
				ELECTRICAL EQUIPMENT: Electrical equipment is			
	NC	N/A		laterally braced to the structure.	13.7.8	A.7.12.12	
c	NC	N/A	U	HR—not required; LS—not required; PR—H. CONDUIT COUPLINGS: Conduit greater than 2.5 in.	15.7.0	A.7.12.12	
				(64 mm) trade size that is attached to panels,			
				cabinets, or other equipment and is subject to			
				relative seismic displacement has flexible couplings			
				or connections.			
Pipin	-				12 7 2	M7122	
c	NC	N/A	U	HR—not required; LS—not required; PR—H. FLEXIBLE COUPLINGS: Fluid and gas piping has	13.7.3 13.7.5	A.7.13.2	
				flexible couplings.	15.7.5		
С	NC	N/A	U	HR—not required; LS—not required; PR—H. FLUID	13.7.3	A.7.13.4	
				AND GAS PIPING: Fluid and gas piping is anchored	13.7.5		
				and braced to the structure to limit spills or leaks.			
C	NC	N/A	U	HR—not required; LS—not required; PR—H. C-	13.7.3	A.7.13.5	
				CLAMPS: One-sided C-clamps that support piping larger than 2.5 in. (64 mm) in diameter are restrained.	13.7.5		
c	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.3	A.7.13.6	
				PIPING CROSSING SEISMIC JOINTS: Piping that crosses	13.7.5	/./.15.0	
				seismic joints or isolation planes or is connected to			
				independent structures has couplings or other details			
				to accommodate the relative seismic displacements.			
Ducts					10.7.6		
c	NC	N/A	U	HR—not required; LS—not required; PR—H . DUCT BRACING: Rectangular ductwork larger than 6 ft ² (0.56	13.7.6	A.7.14.2	
				m ²) in cross-sectional area and round ducts larger			
				than 28 in. (711 mm) in diameter are braced. The			
				maximum spacing of transverse bracing does not			
				exceed 30 ft (9.2 m). The maximum spacing of			
				longitudinal bracing does not exceed 60 ft (18.3 m).	10 7 6		
c	NC	N/A	U	HR—not required; LS—not required; PR—H. DUCT SUPPORT: Ducts are not supported by piping or	13.7.6	A.7.14.3	
				electrical conduit.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.6	A.7.14.4	
				DUCTS CROSSING SEISMIC JOINTS: Ducts that cross			
				seismic joints or isolation planes or are connected to			
				independent structures have couplings or other details to accommodate the relative seismic			
				displacements.			
Eleva	tors			F			
с	NC	N/A	U	HR—not required; LS—H; PR—H. RETAINER	13.7.11	A.7.16.1	
				GUARDS: Sheaves and drums have cable retainer guards.			
С	NC	N/A	U	HR—not required; LS—H; PR—H. RETAINER PLATE:	13.7.11	A.7.16.2	
				A retainer plate is present at the top and bottom of			
				both car and counterweight.			

Legend: C = Compliant, NC = Noncompliant, N/A = Not Applicable, U = Unknown

C N/C N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.3 Image: Im		Project Name Project Number					
Image: Section of the section section of the sectin the section of the section of the section o						- ,	
C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.4 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.4 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.4 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.4 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not required; PR-H. 13.7.11 A.7.16.5 C NC N/A U HR-not required; LS-not	С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16.3
SEISMIC SWITCH: Elevators capable of operating at speeds of 150 ft/min (0.30 m/min) or faster are equipped with seismic switches that meet the requirements of ASME A17.1 or have trigger levels set to 20% of the acceleration of gravity at the base of the structure and 50% of the acceleration of gravity in other locations. C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.6 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.6 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; PR—H. 13.7.11 A.7.16.7					components that are part of the elevator system are		
Speeds of 150 ft/min (0.30 m/min) or faster are equipped with seismic switches that meet the requirements of ASME A17.1 or have trigger levels set to 20% of the acceleration of gravity at the base of the structure and 50% of the acceleration of gravity in other locations. C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.6 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 BRACKETS: The brackets that tie the car rails and the counterweight rails to the structure are sized in accordance with ASME a17.1. A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 BRACKETS: The brackets that tie the car rails and the counterweight rails to the structure are sized in accordance with ASME A17.1. A.7.16.7 BRA	С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16.4
c NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance with ASME A17.1. A.7.16.7 BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance wit					speeds of 150 ft/min (0.30 m/min) or faster are		
C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.5 Image: Comparison of the structure of					requirements of ASME A17.1 or have trigger levels set		
 SHAFT WALLS: Elevator shaft walls are anchored and reinforced to prevent toppling into the shaft during strong shaking. NC N/A U HR—not required; LS—not required; PR—H. COUNTERWEIGHT RAILS: All counterweight rails and divider beams are sized in accordance with ASME A17.1. C NC N/A U HR—not required; LS—not required; PR—H. N/A U HR—not required; LS—not required; PR—H. BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance with ASME A17.1. C NC N/A U HR—not required; LS—not required; PR—H. BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance with ASME A17.1. C NC N/A U HR—not required; LS—not required; PR—H. SPREADER BRACKET: Spreader brackets are not used to resist seismic forces. C NC N/A U HR—not required; LS—not required; PR—H. GO- N/A VU HR—not required; LS—not required; PR—H. GO- 							
 i.i. i.i. i.i. i.i. reinforced to prevent toppling into the shaft during strong shaking. i.i. N/A i.i. VIA i.i.i. VIA i.i. VIA	С	NC	N/A	U		13.7.11	A.7.16.5
C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.6 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.6 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.7 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.8 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.8 C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.9					reinforced to prevent toppling into the shaft during		
 COUNTERWEIGHT RAILS: All counterweight rails and divider beams are sized in accordance with ASME A17.1. NC N/A U HR—not required; LS—not required; PR—H. BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance with ASME A17.1. NC N/A U HR—not required; LS—not required; PR—H. HR—not required; LS—not required; PR—H. SPREADER BRACKET: Spreader brackets are not used to resist seismic forces. NC N/A U HR—not required; LS—not required; PR—H. 60- N/A U HR—not required; LS—not required; PR—H. GO- 	с	NC	N/A	U		13.7.11	A.7.16.6
 BRACKETS: The brackets that tie the car rails and the counterweight rail to the structure are sized in accordance with ASME A17.1. NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.8 SPREADER BRACKET: Spreader brackets are not used to resist seismic forces. N/A U HR—not required; LS—not required; PR—H. GO- 13.7.11 A.7.16.9 					COUNTERWEIGHT RAILS: All counterweight rails and divider beams are sized in accordance with ASME	\bigcirc	
C NC N/A U HR—not required; LS—not required; PR—H. 13.7.11 A.7.16.8 Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the structure are sized in accordance with ASME A17.1. Image: Comparison of the and the accordance with ASM	С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16.7
SPREADER BRACKET: Spreader brackets are not used to resist seismic forces. NC N/A U HR—not required; LS—not required; PR—H. GO- 13.7.11 A.7.16.9					counterweight rail to the structure are sized in		
L L to resist seismic forces. C NC N/A U HR—not required; LS—not required; PR—H. GO- 13.7.11 A.7.16.9	С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16.8
	С	NC	N/A	U		13.7.11	A.7.16.9
Performance Level: HR = Hazards Reduced, LS = Life Safety, and PR = Position Retention.							

^{*a*} Performance Level: HR = Hazards Reduced, LS = Life Safety ^{*b*} Level of Seismicity: L = Low, M = Moderate, and H = High.

Redmond School District John Tuck Elementary School Seismic Evaluation

Appendix C: Schematic Seismic Retrofit Drawings



JOHN TUCK ELEMENTARY SCHOOL SEISMIC RETROFIT

PRELIMINARY DESIGN

REDMOND SCHOOL DISTRICT 209 NW 10TH ST. REDMOND, OR 97756





24 Main Street, Suite 2, Oregon Ci

REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND, OR 97756

JOHN TUCK ELEMENTARY SCHOOL SEISMIC RETROFIT

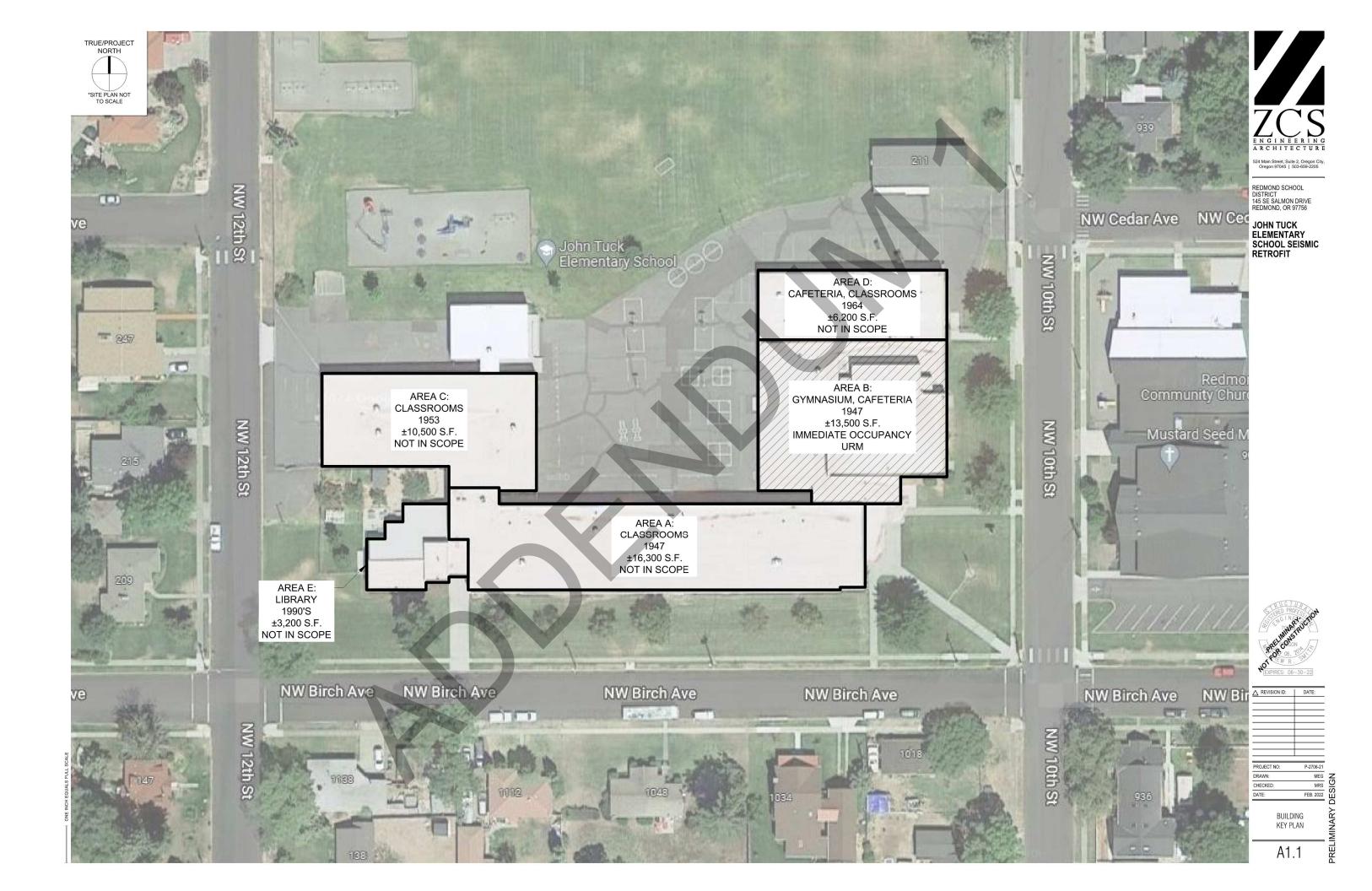


REVISION ID:	DATE:
PROJECT NO:	P-2706-21
DRAWN:	MEG
CHECKED:	MRS
DATE:	FEB. 2022

DESIGN IARY REL



G0.0



- STRUCTURAL REPAIRS: S1. PROVIDE A COMPLETE, WELL-DEFINED LOAD PATH BY INSTALLING NEW ELEM AND CONNECTIONS AS NEEDED TO TRANSFER INERTIAL FORCES FROM A ELEMENTS OF THE BUILDING TO THE FOUNDATION.
- ELEMENTS OF THE BUILDING TO THE FOUNDATION.
 S2. PROVIDE SEISMIC ISOLATION JOINT TO AVOID POUNDING OF THE TALLER STRUCTURE INTO THE LOWER STRUCT PROVIDE ALL NEW GRAVITY FRAMING A LATERAL RESISTING ELEMENTS AS NECESSARY TO PROVIDE BUILDING SEPARATION.
 PROVIDE 2x FRAMED SHEAR WALL
 S3. PROVIDE NEW VERTICAL LATERAL RESISTING ELEMENTS.
 S4. INSTALL NEW OUT-OF-PLANE ANCHORA S5. INSTALL NEW OUT-OF-PLANE ANCHORA S6. INSTALL NEW HARDWARE FOR TRANSF OF SEISMIC FORCES FROM DIAPHRAGN SHEAR WALLS.
 S7. INSTALL NEW WOOD FRAMED SHEAR WALLS WITH STITCH TIES TO SUPPORT EXISTING MANSORY WALLS FOR OUT O PLANE FORCES
 S8. PROVIDE NEW CONTINUOUS CROSS TIE BETWEEN DIAPHRAGM CHORDS.
 S9. INSTALL NEW PLYWOOD DIAPHRAGM

- - S9. INSTALL NEW PLYWOOD DIAPHRAGM SHEATHING.
 - S10. INSTALL NEW PLYWOOD DIAPHRAGM
 - SHEATHING. S11. S12. INSTALL NEW OUT-OF-PLANE ANCHOR INSTALL NEW SECONDARY SUPPORT FOR VERTICAL LOAD CARRYING FRAMING
 - ELEMENTS. STRENGTHEN EXISTING BOWSTRING TRUSSES FOR PRESCRIBED SEISMIC S13. LOADS

	NON-S	TRUCTURAL REPAIRS:
ED	N1.	BRACE PIPING OR DUCTWORK CONVEYING
MENTS		HAZARDOUS MATERIALS.
	N2.	INSTALL FLEXIBLE COUPLINGS FOR
ALL		DUCTWORK AND PIPING CONTAINING
		HAZARDOUS MATERIAL, INCLUDING
		NATURAL GAS PIPING.
ТО	N3.	BRACE UNREINFORCED MASONRY OR
		HOLLOW-CLAY TILE PARTITIONS.
CTURE.	N4.	INDEPENDENTLY BRACE THE TOPS OF
G AND		MASONRY OR HOLLOW-CLAY TILE
		PARTITIONS.
	N5.	SECURE EXISTING MASONRY VENEER WITH
		NEW STITCH TIES.
	N6.	INSTALL WOOD FRAMED WALLS WITH
		STITCH TIES TO SUPPORT EXISTING
		MASONRY WALLS FOR OUT-OF-PLANE
RAGE.		FORCES.
RAGE.	N7.	BRACE EXISTING BACKUP WALL WITH NEW
SFER		ADJACENT WALL FRAMING.
GM TO	N8.	INSTALL WOOD FRAMED WALLS WITH
		STITCH TIES TO SUPPORT EXISTING
र		MASONRY WALLS FOR OUT-OF-PLANE
RT		FORCES.
ΓOF	N9.	PROVIDE BRACING OF PARAPETS OR
		CORNICES.
TIES	N10.	PROVIDE ANCHORAGE OF APPENDAGES
		TO THE STRUCTURE.
1	N11.	ANCHOR CONTENTS TO THE STRUCTURE.
_	N12.	INSTALL FLEXIBLE COUPLINGS FOR FLUID
1		AND GAS PIPING.
	N13.	ANCHOR AND BRACE FLUID AND GAS
RAGE.		PIPING TO THE STRUCTURE.
r for		



524 Main Street, Suite 2, Oregon City, Oregon 97045 | 503-659-2205

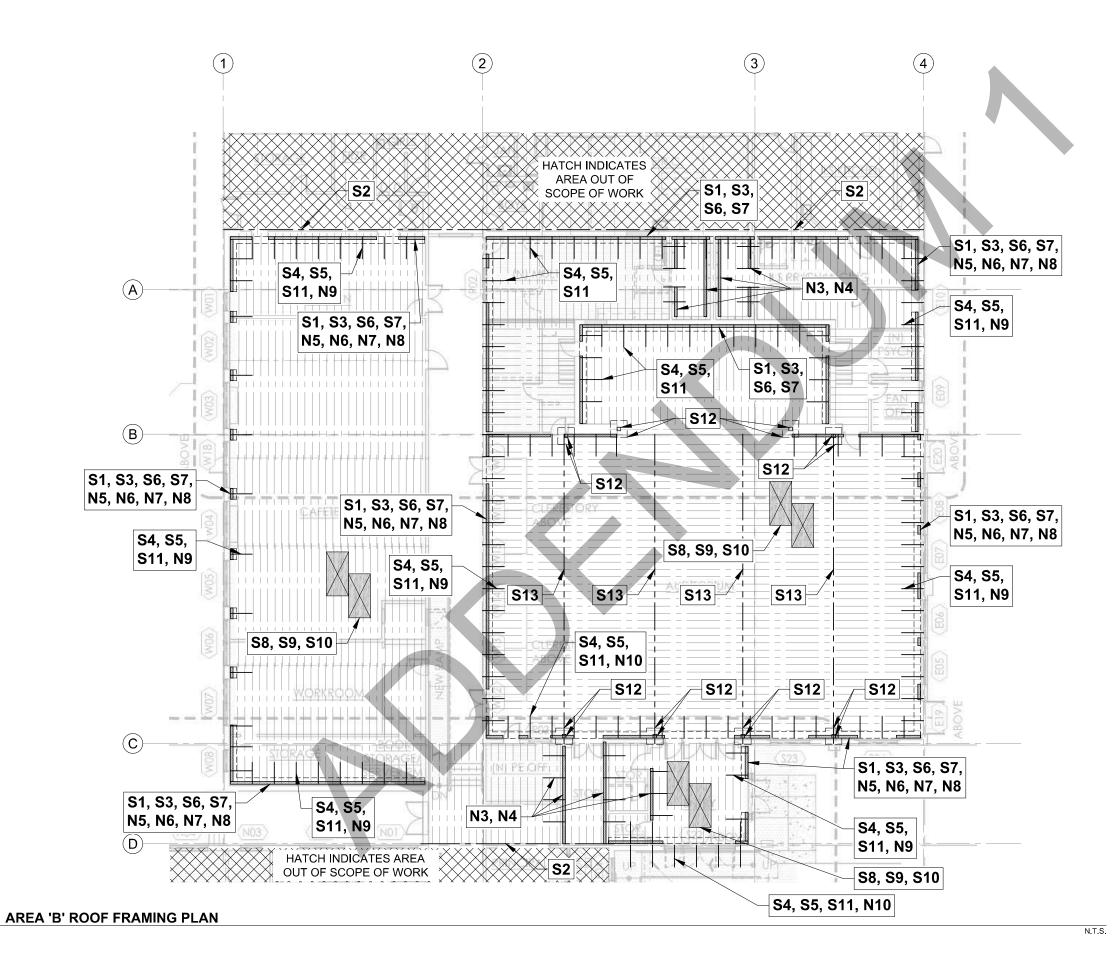
REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND, OR 97756

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JOHN TUCK ELEMENTARY SCHOOL SEISMIC RETROFIT



	DATE:				
PROJECT NO:	P-2706-21				
DRAWN:	MEG	DESIGN			
CHECKED:	MRS	ĕ			
DATE:	FEB. 2022	Щ			
REPAIR KEY NOTES					
S1.1					

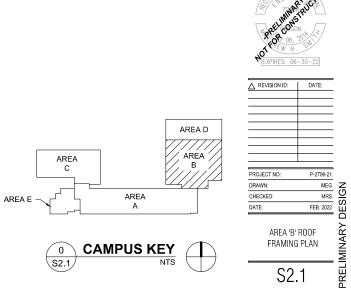


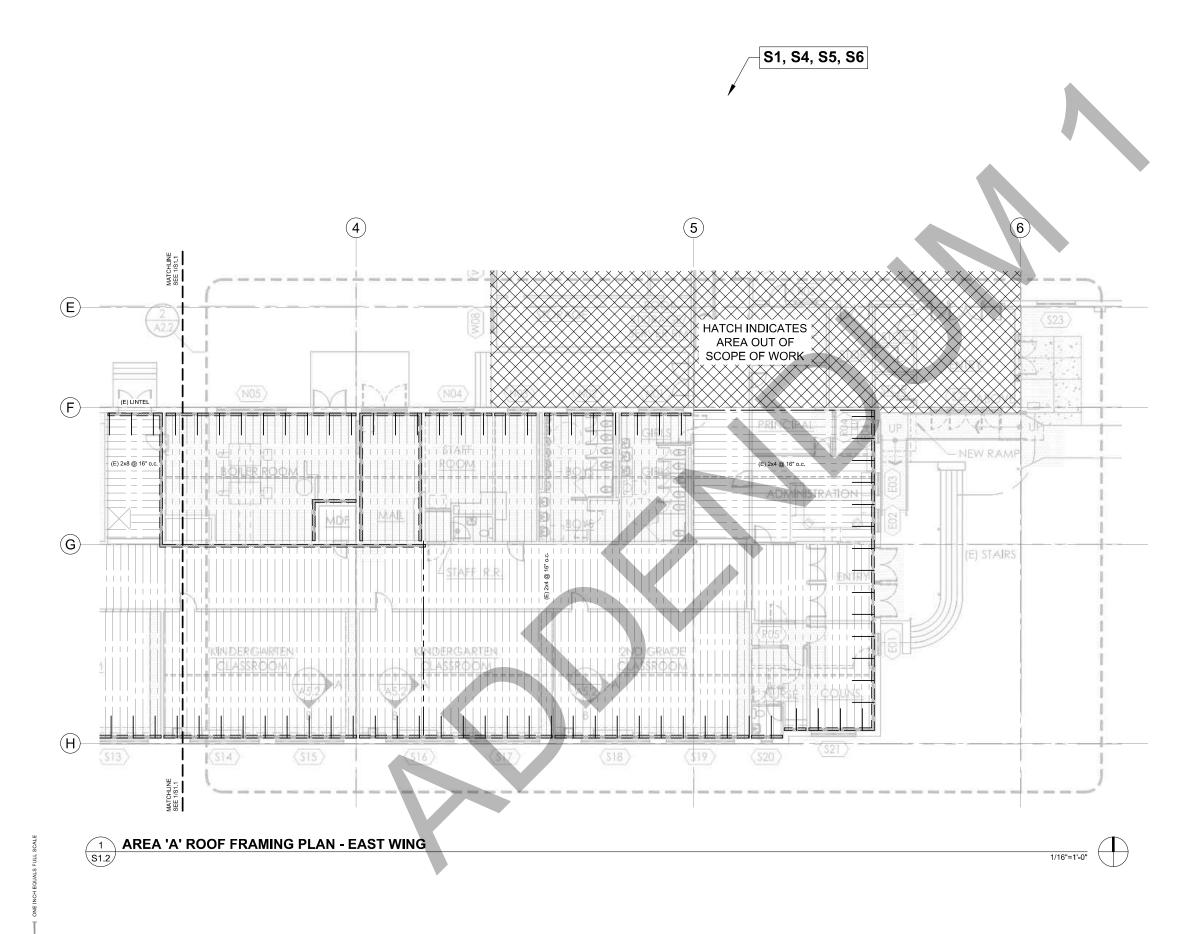


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REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND, OR 97756

JOHN TUCK ELEMENTARY SCHOOL SEISMIC RETROFIT



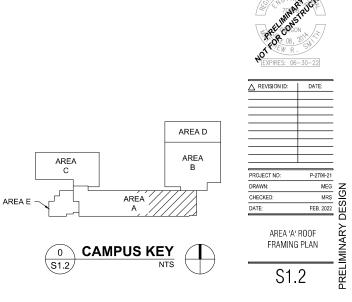




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REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND, OR 97756

JOHN TUCK ELEMENTARY SCHOOL SEISMIC RETROFIT



Redmond School District John Tuck Elementary School Seismic Evaluation

Appendix D: Geotechnical Information





OSHPD

Latitude, Longitude: 44.27793421408763, -121.17956523856027

Dog park Dog park Dog park Dog park Date Design Code Reference Document Custom Probability Site Class	Elementary School	Redmond B Com 11/3/2021, 1:49:42 PM ASCE41-17 D - Default (See Section 11.4.3)	Black Bea Diner Redmon
Туре	Description		Value
Hazard Level			BSE-2N
SS	spectral response (0.2 s)		0.361
S ₁	spectral response (1.0 s)		0.187
S _{XS}	site-modified spectral response (0.2 s)		0.545
S _{X1}	site-modified spectral response (1.0 s)		0.416
Fa	site amplification factor (0.2 s)		1.511
F _v	site amplification factor (1.0 s)		2.227
ssuh	max direction uniform hazard (0.2 s)		0.4
crs	coefficient of risk (0.2 s)		0.903
ssrt	risk-targeted hazard (0.2 s)		0.361
ssd	deterministic hazard (0.2 s)		1.5
s1uh	max direction uniform hazard (1.0 s)		0.212
cr1	coefficient of risk (1.0 s)		0.88
s1rt	risk-targeted hazard (1.0 s)		0.187
s1d	deterministic hazard (1.0 s)		0.6
Туре	Description		Value
Hazard Level			BSE-1N
S _{XS}	site-modified spectral response (0.2 s)		0.364
S _{X1}	site-modified spectral response (1.0 s)		0.277

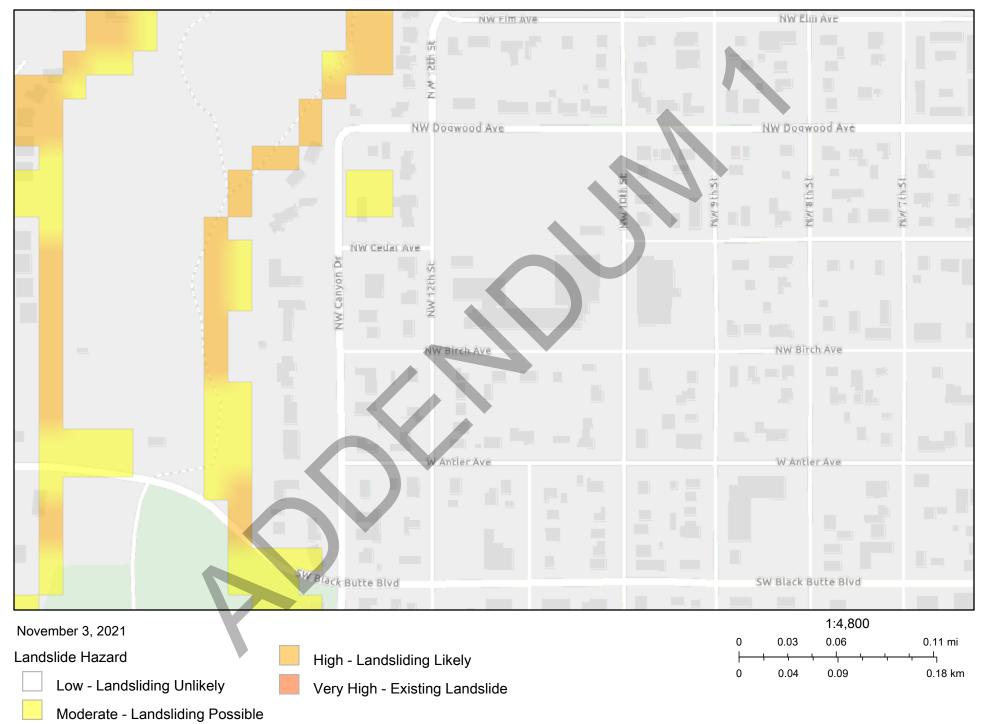
Туре	Description	Value
Hazard Level		BSE-2E
SS	spectral response (0.2 s)	0.25
S ₁	spectral response (1.0 s)	0.129
S _{XS}	site-modified spectral response (0.2 s)	0.4
S _{X1}	site-modified spectral response (1.0 s)	0.303
f _a	site amplification factor (0.2 s)	1.6
f _v	site amplification factor (1.0 s)	2.342

Туре	Description	Value
Hazard Level		BSE-1E
SS	spectral response (0.2 s)	0.099
S ₁	spectral response (1.0 s)	0.046
S _{XS}	site-modified spectral response (0.2 s)	0.159
S _{X1}	site-modified spectral response (1.0 s)	0.11
F _a	site amplification factor (0.2 s)	1.6
F _v	site amplification factor (1.0 s)	2.4
Туре	Description	Value
Hazard Level		TL Data
T-Sub-L	Long-period transition period in seconds	16

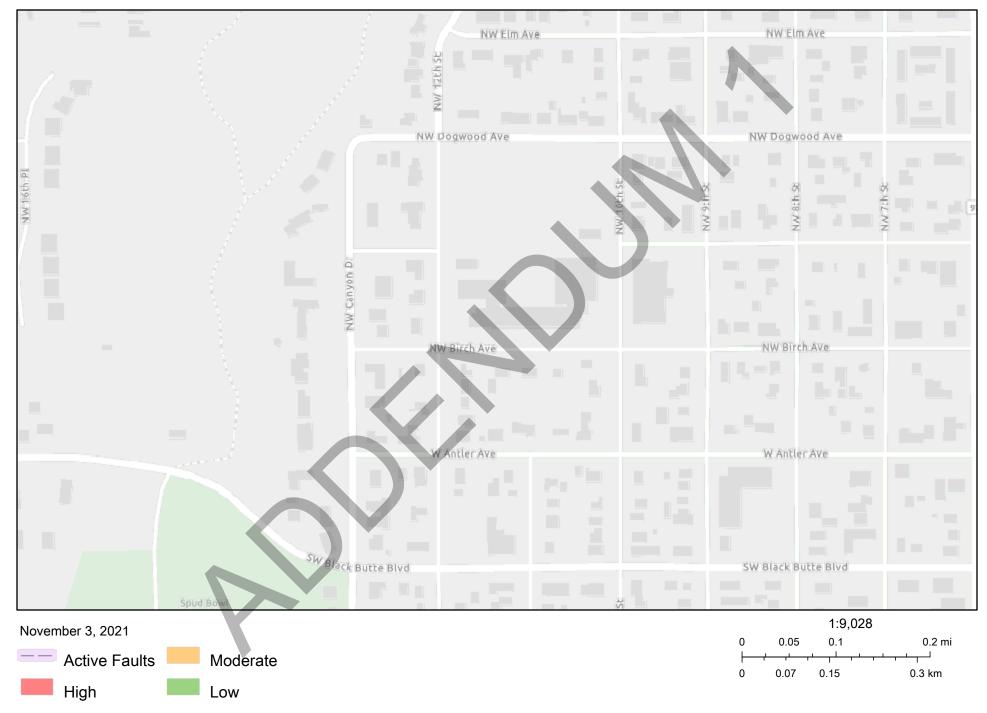
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John Tuck ES DOGAMI Landslide



Redmond DOGAMI Liquefaction



Redmond School District John Tuck Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix E: Construction Cost Estimate Worksheets

ZCS

ENGINEER'S OPINIC	ENGINEER'S OPINION OF PROBABLE COST - JOHN TUCK ELEMENTARY SCHOOL SEISMIC REHABILITATION							
SUMMARY								
Description	Deficiencies (Ref. Seismic Evaluation Report Sec. 7.0)	Quantity	Units	Unit Price		Total Price for		
		GENERAL CONDITI	ONS					
General Conditions Preconstruction Services		10% 2%	%		\$ \$	145,060.00 29,012.00		
Escalation Bonding & Insurance Contractor Profit & Overhead		7% 3% 5%	% % %		\$ \$ \$	113,727.04 48,740.16 81,233.60		
			-	I Conditions Subtotal	\$	417,772.80		
		Non-Structural Elem						
Misc MEP Misc Non-Structural New Restroom	N1, N2, N12, N13 N11 N3, N4	1 1 1	Lump Sum Lump Sum EA	\$ 93,600.00 \$ 37,500.00 \$ 20,000.00	\$ \$ \$	93,600.00 37,500.00 20,000.00		
			No	on-Structural Subtotal	\$	151,100.00		
	Const	ruction Cost Per Bu	ilding Part					
			Buil	ding Part 'A' Subtotal	\$	-		
			Buil	ding Part 'B' Subtotal	\$	1,299,500.00		
				ding Part 'C' Subtotal		-		
			Buil	ding Part 'D' Subtotal				
			Sub-Total C	onstruction Cost	\$	1,868,400.00		
			Contingend		\$	186,840.00		
				onstruction Cost	\$	2,055,240.00		
		Cost Estimate Sum	marv					
Engineering Architectural Consulting Structural / Rehabilitation Engineering Geotechnical Consulting Materials Testing for Design URM Tier 3 Analysis		$\boldsymbol{\cdot}$		\$ 30,800.00 \$ 226,100.00 \$ 10,300.00 \$ 10,300.00 \$ 5,000.00	\$	282,500.00		
Construction Management Construction Sub-Total Construction Cost Special Inspection Services for Construction Permitting Fees		\sim		\$ 1,868,400.00 \$ 10,300.00 \$ 61,700.00	\$	61,700.00 1,940,400.00		
Relocation of FF&E					\$	28,000.00		
Contingency			Total Drainat Fundin	n Deguirement	\$	186,840.00		
			Total Project Fundin	g kequirement	\$	2,499,440.00		

ENGINEER'S C	OPINION OF PROBABLE COST - J	OHN TUCK ELEMEN	ITARY SCHOOL SEISMI		ΓΙΟΝ
	BU	ILDING PART - 'B'			
Description	Deficiencies (Ref. Seismic Evaluation Report Sec. 7.0)	Quantity	Units	Unit Price	Total Price for Construction Item
	Demolitio	n & Asbestos Abate	ment	•	
Soft Demolition Abatement Hard Demolition	S1, S2, S3, S4, S5, S6, S7, S8, S9, S10, S11 S1, S2, S3, S4, S5, S6, S7, S8, S9, S10, S11 S3, S7, S12	13500 13500 1900	Square Foot Square Foot Square Foot	\$ 2.00 \$ 5.00 \$ 20.00	\$ 67,500.00
			Demolition 8	Asbestos Subtotal	\$ 132,500.00
	Foundation / Flo	oor Strengthening Co	onstruction		
Gym Floor Patch / Replacement Spread Footings for Columns / Holdown Shear Wall Footings - Wood Walls Floor Finish Patch / Replacement	\$3, \$7, \$12 \$12 \$3, \$7 \$3, \$7, \$12	5400 8 625 4400	Square Foot Each Linear Foot Square Foot	\$ 13.00 \$ 4,000.00 \$ 300.00 \$ 7.00	\$ 32,000.00
			Found	ation Level Subtota	\$ 320,500.00
	Wall Stre	ngthening Construc	tion		
New 2x Framed Shear Walls Interior Wall Finish Repair Painting Masonry Ties Heavy Steel Columns	\$3, \$7 \$3, \$7 \$3, \$7 \$3, \$7 \$3, \$7, N3, N4, N5, N6, N7, N8 \$12	10900 10900 10900 10900 8	Square Foot Square Foot Square Foot Square Foot EA	\$ 10.00 \$ 200 \$ 3.00 \$ 20.00 \$ 7,500.00	\$ 21,800.00 \$ 32,700.00 \$ 218,000.00
				engthening Subtotal	\$ 441,500.00
		engthening Construct			
Existing Truss Strengthening Diaphragm Attachments - Out-of-Plane Diaphragm Attachments - In-Plane Shear Seismic Isolation from Adjacent Building Parapet Bracing Fold Back Existing Roofing for Diaphragm New Ceiling Sheathing Ceiling Repair Painting	513 S4, 55, 511 S1, 56 S2 N9 S2, N9 S8, 59, 510 S1, 52, 54, 55, 57, 58, 59, S10, 511 S1, 52, 54, 55, 57, 58, 59, S10, 511	4 770 650 164 200 800 13500 13500 13500 13500	EA Linear Foot Linear Foot Linear Foot Square Foot Square Foot Square Foot Square Foot	\$ 30,000.00 \$ 50.00 \$ 20.00 \$ 400.00 \$ 65.00 \$ 8.00 \$ 5.00 \$ 3.00 \$ 3.00	\$ 38,500.00 \$ 13,000.00 \$ 65,600.00 \$ 13,000.00 \$ 6,400.00 \$ 67,500.00 \$ 40,500.00
				engthening Subtotal	
		Bui	Iding Part 'B' - Total Cor	nstruction Cost	\$ 1,299,500.00

Redmond School District John Tuck Elementary School Seismic Evaluation

Appendix F: Rapid Visual Screening

Rapid Visual Screening of Buildings for Potential Seismic Hazards

FEMA P-154 Data Collection Form

							Add	dress:	209 N	W 10th	n Street							
								_	Redm	ond, O	R			Z	lip:	97756	;	
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			100	18	100	10	Geo	ologic H						-	/No/DNK	Surf R	upt.: Yes	
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A STATEMENT	- 10			2.4	-	100	Ext	erior Fal	ling 🧄		Inbraced						leavy Ven	
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and the second se	01/1					1.000												
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	Do Not	W1	W1A	SCO W2	RE, MO	S2	53 S3			r		RE, 5/ C3	L1 PC1	PC2	DM4	DM2		МН
FEMA BUILDING TYPE	Know	VVI	WIA	VV2	(MRF)	(BR)	(LM)	(RC SW)	S5 (URM INF)	C1 (MRF)	(SW)	URM INF)	(TU)	PG2	RM1 (FD)	RM2 (RD)	URM	
Basic Score		3.6	3.2	2.9	2.1	2.0	2.6	2.0	1.7	1.5	2.0	1.2	1.6	1.4	1.7	1.7	1.0	1.5
Severe Vertical Irregularity, VL1		-1.2	-1.2	-1.2		-1.0	-1.1	-1.0	-0.8	-0.9	-1.0	-0.7	-1.0	-0.9	-0.9	-0.9	-0.7	NA
Moderate Vertical Irregularity, V_{L1}		-0.7	-0.7	-0.7	-0.6	-0.6	-0.7	-0.6	-0.5	-0.5	-0.6	-0.4	-0.6	-0.5	-0.5	-0.5	-0.4	NA
Plan Irregularity, <i>P</i> _{L1} Pre-Code		-1.1 -1.1	-1.0 -1.0	-1.0 -0.9	-0.8 -0.6	-0.7 -0.6	-0.9 -0.8	-0.7 -0.6	-0.6 -0.2	-0.6 -0.4	-0.8 -0.7	-0.5 -0.1	-0.7 -0.5	-0.6 -0.3	-0.7 -0.5	-0.7 -0.5	-0.4	NA -0.1
Post-Benchmark		1.6	1.9	2.2	-0.0	1.4	1.1	-0.0	NA	1.9	2.1	NA	2.0	2.4	2.1	2.1	NA	1.2
Soil Type A or B		0.1	0.3	0.5	0.4	0.6	0.1	0.6	0.5	0.4	0.5	0.3	0.6	0.4	0.5	0.5	0.3	0.3
Soil Type E (1-3 stories)		0.2	0.2	0.1	-0.2	-0.4	0.2	-0.1	-0.4	0.0	0.0	-0.2	-0.3	-0.1	-0.1	-0.1	-0.2	-0.4
Soil Type E (> 3 stories)		-0.3	-0.6	-0.9	-0.6	-0.6	NA	-0.6	-0.4	-0.5	-0.7	-0.3	NA	-0.4	-0.5	-0.6	-0.2	NA
Minimum Score, S _{MIN}		1.1	0.9	0.7	0.5	0.5	0.6	0.5	0.5	0.3	0.3	0.3	0.2	0.2	0.3	0.3	0.2	1.0
FINAL LEVEL 1 SCORE, SL	ı ≥ Smin:	0.6;	THERE	EFOR	RE FEMA	154 C		PSE PO	TENTIA	AL IS ~	HIGH (>	>10%)						
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Interior: None			X Ente	ered	Detailed	Structu	ral Evalu	uation?			es, unkno				r other bu	uilding		
Drawings Reviewed: X Yes Soil Type Source: NONE		10						nless SL2	>		es, score					-		
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Contact Person: JOSEP LEVEL 2 SCREENING Yes, Final Level 2 Score, SL Nonstructural hazards? Where info Legend: MRF = M	H GIPN PERFC 2 Yes	ER DRME	⊠ N □ N e verifie	lo d, scr RC = R	Geole Signi the s	ogic haz ficant da tructura	amage/de I system he follov	eterioratio	on to S T = Esti = Unreinfo	☐ Y ⊠ N de ☐ N	es, nonst lo, nonstru etailed ev lo, no non or unrelia	ructural h uctural ha aluation structura ble data MH	nazards azards e is not ne al hazard <u>OR</u>	identified exist that cessary ls identified DNK = D ctured Ho	that sho may requ ed [o Not Kr using Fi	uld be e uire mitig DNK now D = Flexit	valuated ation, but	gm

Rapid Visual Screening of Buildings for Potential Seismic Hazards FEMA P-154 Data Collection Form

							Add	ress:	209 N	W 10th	Street							
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	SKE	тсн					М	Addition	al sketche	es or cor	nments o	n separa	ate page					
		B/	ASIC	sco	RE, MO	DIFIER												
FEMA BUILDING TYPE	Do Not	W1	W1A	W2	S1	S2	53	S4	S 5	C1	C2	C3	PC1	PC2	RM1	RM2	URM	МН
	Know				(MRF)	(BR)	(LM)	(RC	(URM	(MRF)	(SW)	(URM INF)	(TU)		(FD)			
	KIIOW															(RD)		
Basic Score	KIIOW	3.6	3.2	2.9	2.1	2.0	2.6	SW) 2.0	INF)	1.5	2.0	1.2	1.6	1.4	1.7	(RD) 1.7	1.0	1.5
Severe Vertical Irregularity, VL1		-1.2	-1.2	-1.2	-1.0	-1.0	-1.1	ŚW) 2.0 -1.0	INF) 1.7 -0.8	-0.9	-1.0	1.2 -0.7	-1.0	-0.9	-0.9	1.7 -0.9	-0.7	NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1}		-1.2 -0.7	-1.2 -0.7	-1.2 -0.7	-1.0 -0.6	-1.0 -0.6	-1.1 -0.7	ŚW) 2.0 -1.0 -0.6	INF) 1.7 -0.8 -0.5	-0.9 -0.5	-1.0 -0.6	1.2 -0.7 -0.4	-1.0 -0.6	-0.9 -0.5	-0.9 -0.5	1.7 -0.9 -0.5	-0.7 -0.4	NA NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1}		-1.2 -0.7 -1.1	-1.2 -0.7 -1.0	-1.2 -0.7 -1.0	-1.0 -0.6 -0.8	-1.0 -0.6 -0.7	-1.1 -0.7 -0.9	ŚW) 2.0 -1.0 -0.6 -0.7	INF) 1.7 -0.8 -0.5 -0.6	-0.9 -0.5 -0.6	-1.0 -0.6 -0.8	1.2 -0.7 -0.4 -0.5	-1.0 -0.6 -0.7	-0.9 -0.5 -0.6	-0.9 -0.5 -0.7	1.7 -0.9 -0.5 -0.7	-0.7 -0.4 -0.4	NA NA NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1}		-1.2 -0.7	-1.2 -0.7	-1.2 -0.7	-1.0 -0.6	-1.0 -0.6	-1.1 -0.7	ŚW) 2.0 -1.0 -0.6	INF) 1.7 -0.8 -0.5	-0.9 -0.5	-1.0 -0.6	1.2 -0.7 -0.4	-1.0 -0.6	-0.9 -0.5	-0.9 -0.5	1.7 -0.9 -0.5	-0.7 -0.4	NA NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code		-1.2 -0.7 -1.1 -1.1	-1.2 -0.7 -1.0 -1.0	-1.2 -0.7 -1.0 -0.9	-1.0 -0.6 -0.8 -0.6	-1.0 -0.6 -0.7 -0.6	-1.1 -0.7 -0.9 -0.8	\$W) 2.0 -1.0 -0.6 -0.7 -0.6	INF) 1.7 -0.8 -0.5 -0.6 -0.2	-0.9 -0.5 -0.6 -0.4	-1.0 -0.6 -0.8 -0.7	1.2 -0.7 -0.4 -0.5 -0.1	-1.0 -0.6 -0.7 -0.5	-0.9 -0.5 -0.6 -0.3	-0.9 -0.5 -0.7 -0.5	1.7 -0.9 -0.5 -0.7 -0.5	-0.7 -0.4 -0.4 0.0	NA NA NA -0.1
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories)		-1.2 -0.7 -1.1 -1.1 1.6	-1.2 -0.7 -1.0 -1.0 1.9	-1.2 -0.7 -1.0 -0.9 2.2	-1.0 -0.6 -0.8 -0.6 1.4	-1.0 -0.6 -0.7 -0.6 1.4	-1.1 -0.7 -0.9 -0.8 1.1	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA	-0.9 -0.5 -0.6 -0.4 1.9	-1.0 -0.6 -0.8 -0.7 2.1	1.2 -0.7 -0.4 -0.5 -0.1 NA	-1.0 -0.6 -0.7 -0.5 2.0	-0.9 -0.5 -0.6 -0.3 2.4	-0.9 -0.5 -0.7 -0.5 2.1	1.7 -0.9 -0.5 -0.7 -0.5 2.1	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2	NA NA -0.1 1.2
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)		-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.4	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, S_{MIN}		-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2	NA NA -0.1 1.2 0.3 -0.4
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)		-1.2 -0.7 -1.1 1.6 0.1 0.2 -0.3 1.1	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 -0.4 -0.5	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, S_{MIN}		-1.2 -0.7 -1.1 1.6 0.1 0.2 -0.3 1.1	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 -0.4 -0.5	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%)	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, S_{MIN} FINAL LEVEL 1 SCORE, S_L EXTENT OF REVIEW Exterior: Parti	L1≥ Smin: ial ⊠ /	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6;	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO	NF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 NL IS ~	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (>	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, St EXTENT OF REVIEW Exterior: Parti Interior: Non	L1≥ Smin: ial ⊠ ¢ e ⊠ \	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; /isible	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 E FEMA	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CCC R HAZ/ e Hazard	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS s That 1	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger <i>I</i>	NF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 NL IS ~ ACT Detail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (>	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF tural Ev	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, St EXTENT OF REVIEW Exterior: Parti Interior: Nonu Drawings Reviewed: X Yes	L1≥ Smin: ial ⊠ /	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; /isible	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 E FEMA OTHEI Are Ther Detailed Poun	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS s That T al Evalu ntial (un	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation?	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 L IS ~ ACT Detail D Y(D Y(-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 -10%) EQUIF tural Ev wm FEM less tha	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildin n cut-off	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, St EXTENT OF REVIEW Exterior: Parti Interior: Nonu Drawings Reviewed: X Yes Soil Type Source: NONE	$1 \ge S_{MIN}$ ial $X \not\in A$	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; /isible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 E FEMA OTHEF Are Ther Detailed Detailed	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote ff, if know	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS s That T al Evalu ntial (un n)	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation?	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~ ACT Detail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 -10%) EQUIF tural Ev wm FEM less tha	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildin n cut-off	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, St EXTENT OF REVIEW Exterior: Parti Interior: Non- Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source:	L1≥ Smin: ial X / e X \ DOGAM	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 E FEMA OTHEF Are Ther Detailed Poun cut-o Fallir	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote ff, if know g hazarda	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS s That T al Evalu ntial (un n)	\$W) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation?	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 L IS ~ ACT Detail D Ya D Ya N Ya N N	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other o	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF tural Ev wm FEM less tha hazards	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildin n cut-off present	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SU EXTENT OF REVIEW Exterior: Parti Interior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF	L1 ≥ Smin: ial X / e X 1 DOGAM PH GIPN	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer X Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 E FEMA OTHEI Are Ther Detailed Poun cut-o E Fallir build	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote ff, if know g hazarda	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS s That T al Evalu ntial (un n) s from ta	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation? lless SL2 aller adja	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AC T Detail Detail Y(Y) V(V) V(V) V(Detail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other o ed Nonst	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 -10%) EQUIF tural Ev wwn FEM less tha hazards	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildin n cut-off present	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, St EXTENT OF REVIEW Exterior: Parti Interior: Non- Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source:	L1 ≥ Smin: ial X / e X 1 DOGAM PH GIPN	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer X Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA OTHE! Are Ther Detailed Detailed Detailed Detailed Detailed Signi	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ HAZ HAZ Hazard Structura ding pote ff, if know g hazard: ng ogic haza ficant dar	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP DLLAP S That T al Evalu ntial (un n) s from ta rds or S nage/de	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO' Irigger A ation? illess SL2 aller adja oil Type	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA Cent F	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 (L IS ~ Detail Y(X N(Detail C Y(S N(Detail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other lo o ed Nonst	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF tural Ev wm FEM less tha hazards	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation A buildin n cut-off present I Evalua hazards	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type of tion Rec identified	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SU EXTENT OF REVIEW Exterior: Parti Interior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF	L1≥ Smin: ial X A e X V DOGAM PH GIPN PERFC	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer X Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA OTHE! Are Ther Detailed Detailed Detailed Detailed Detailed Signi	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote ff, if know g hazard: ng ogic haza	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP DLLAP S That T al Evalu ntial (un n) s from ta rds or S nage/de	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO' Irigger A ation? illess SL2 aller adja oil Type	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA Cent F	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 (L IS ~ ACT Detail \Box Y(\Box Y(\Box N(Detail \Box Y(\Box N(Detail) \Box Y(\Box N(Detail)	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other lo o ed Nonst es, nonstru o, nonstru	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF tural Ev wwn FEM less tha hazards tructural h uctural h	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation A buildin n cut-off present I Evalua nazards azards e	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type of tion Rec identified exist that i	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SA EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF LEVEL 2 SCREENING Yes, Final Level 2 Score, SA	L1≥ Smin: ial X A e X V DOGAM PH GIPN PERFC	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA OTHE! Are Ther Detailed Detailed Detailed Detailed Detailed Signi	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ HAZ HAZ Hazard Structura ding pote ff, if know g hazard: ng ogic haza ficant dar	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP DLLAP S That T al Evalu ntial (un n) s from ta rds or S nage/de	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO' Irigger A ation? illess SL2 aller adja oil Type	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA Cent F	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 ACT Detail D Y4 D Y4 D Y4 D Ptail D Y4 D Y4 D Y4 D Y4 D Y4 D Y4 D Y4 D Y4	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other lo o ed Nonst	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 >10%) EQUIF tural Ev wm FEM less tha hazards tructural l uctural h aluation	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec identified exist that i	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? r other but that sho may requ	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SA EXTENT OF REVIEW Exterior: Parti Interior: Nonu Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF LEVEL 2 SCREENING Yes, Final Level 2 Score, S Nonstructural hazards?	I1 ≥ SMIN: ial X A e X Y DOGAM PH GIPN PERFC 12 Yes	-1.2 -0.7 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No	-1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer X Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR tered	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA Are Ther Detailed Poun cut-o Poun cut-o Fallir build Geolo Signi the s	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard ding pote ff, if know g hazard: ng pogic haza ficant dars	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS S That 1 al Evalu ntial (un n) s from ta rds or S nage/de system	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.5 SE PO' Frigger A ation? aller adja oil Type terioratic	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 TENTIA cent F n to	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~ Detail Y4 Y4 Y4 Y4 Y4 Y4 Detail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other o ed Nonst es, nonstru o, nonstru o, nonstru o, no non	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 -10%) EQUIF tural Ev wm FEM less tha hazards tructural h actural h aluation structura	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation A buildin n cut-off present I Evalua hazards azards e is not ne al hazard	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec identified exist that i	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 onmeno that sho may requ	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch uild be e uire mitig	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SA EXTENT OF REVIEW Exterior: Parti Interior: Nonu Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF LEVEL 2 SCREENING Yes, Final Level 2 Score, S Nonstructural hazards?	I1 ≥ SMIN: ial X A e X Y DOGAM PH GIPN PERFC 12 Yes	-1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 0.6; Visible No 1 ER DRMEI	-1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THER X Aer X Ent	-1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR rial tered	-1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 E FEMA Are Ther Detailed Poun cut-o Poun cut-o Fallir build Geolo Signi the s	-1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 CC R HAZ/ e Hazard Structura ding pote ff, if know g hazard: fig hazard: ng ogic haza ficant dar tructural s	-1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 DLLAP ARDS S That T al Evalu ntial (un n) s from ta rds or S nage/de system	św) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 0.5 SE PO' Trigger A ation? aller adja oil Type terioratic	INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 TENTIA cent F n to	-0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~ ACT Detail OPtail OPtail OPtail OPtail OPtail OPtail	-1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (> ION RI ed Struct es, unkno es, score es, other o ed Nonst es, nonstru o, no nor r unrelia	1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 -10%) EQUIF tural Ev wm FEM less tha hazards tructural h actural h aluation structura ble data	-1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation A buildin n cut-off present I Evalua hazards azards e is not ne al hazard	-0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec identified exist that is cessary Is identified	-0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 or other but that sho may requ ed [o Not Kr	1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch uild be e uire mitig DNK 10W	-0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	NA NA -0.1 1.2 0.3 -0.4 NA 1.0

Rapid Visual Screening of Buildings for Potential Seismic Hazards FEMA P-154 Data Collection Form

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BASIC SCORE, N FEMA BUILDING TYPE Do Not Know W1 W1A W2 W2 S3 (MR)	MODIFIERS, A 1 S2 S3 (BR) (LM)	S4 S5 (RC (URM SW) INF)	C1 (MRF)	C2 (SW) (SW) (URM INF)	L1 PC1 PC2 (TU)	(FD)	(RD)		
BASIC SCORE, N FEMA BUILDING TYPE Do Not Know W1 W1A W2 St (MR Basic Score 3.6 3.2 2.9 2.' Severe Vertical Irregularity, V _{L1} -1.2 -1.2 -1.2 -1.2 -1.2	S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1	S4 S5 (RC (URM	LEVEL 1 (MRF) 1.5	C2 (SW) (URM	L1 PC1 PC2	(FD) 1.7		URM 1.0 -0.7	MH 1.5 NA
BASIC SCORE, N FEMA BUILDING TYPE Do Not Know W1 W1A W2 St (MR Basic Score 3.6 3.2 2.9 2.' Severe Vertical Irregularity, V _{L1} -1.2 -1.2	Image: Non-Section 1 S2 (BR) (LM) S3 (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7	S4 S5 (RC (URN) SW) INF) 2.0 1.7 -1.0 -0.8 -0.6 -0.5	LEVEL 1 (MRF) 1.5 -0.9 -0.5	C2 (SW) C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.5	(FD) 1.7 -0.9 -0.5	(RD) 1.7 -0.9 -0.5	1.0 -0.7 -0.4	1.5 NA NA
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St (MR Basic Score 3.6 3.2 2.9 2.' Severe Vertical Irregularity, V _{L1} -1.2 -1.2 -1.2 -1.2 -1.2 Moderate Vertical Irregularity, V _{L1} -0.7 -0.7 -0.7 -0.7 -0.7 Plan Irregularity, P _{L1} -1.1 -1.0 -1.0 -0. -0.	I S2 S3 (BR) (LM) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9	S4 S5 (RC (URN SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6	LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6	C2 (SW) C3 (URM (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.5 -0.7 -0.6	(FD) 1.7 -0.9 -0.5 -0.7	(RD) 1.7 -0.9 -0.5 -0.7	1.0 -0.7 -0.4 -0.4	1.5 NA NA NA
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.1 Severe Vertical Irregularity, VL1 -1.2 -1.1 -1.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.0 -0.2 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2	I S2 S3 (BR) (LM) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8	S4 S5 (RC (URN) SW) INF) 2.0 1.7 -1.0 -0.8 -0.6 -0.5	LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6	C2 (SW) C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.5	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 -0.5	(RD) 1.7 -0.9 -0.5	1.0 -0.7 -0.4	1.5 NA NA
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.1 Severe Vertical Irregularity, VL1 -1.2 -1.1 -1.0 -0.0 -0.0 -0.1 0.1 0.0 -0.9 -0.1 -0.2 -0.1 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 Soil Type A or B 0.1 0.3 0.5 0.4	I S2 S3 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1	S4 (RC (RC) S5 (URM (INF) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5	LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4	SCORE, S, (SW) C3 (URM (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4	(FD) 1.7 -0.9 -0.5 -0.5 -0.7 2.1 0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5	-0.7 -0.4 -0.4 0.0 NA 0.3	1.5 NA NA -0.1 1.2 0.3
BASIC SCORE, N FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.1 Severe Vertical Irregularity, V_{L1} -1.2 -1.1 -1.0 -0.0 -0.1 0.7 -0.7 -0.7 -0.0 -0.1 0.1 0.3 0.5 0.2 0.2 1.4 -0.1 -0.3 0.5 0.4 0.3 0.5 0.4 0.3 0.5 0.4 0.2 0.2 0.1 -0.2 0.2 0.1 -0.5 -0.5 <t< th=""><th>I S2 S3 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2</th><th>S4 S5 (RC (URM SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4</th><th>LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0</th><th>SCORE, S, (SW) C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2</th><th>L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1</th><th>(FD) 1.7 -0.9 -0.5 -0.5 -0.7 2.1 0.5 -0.1</th><th>(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1</th><th>1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2</th><th>1.5 NA NA -0.1 1.2 0.3 -0.4</th></t<>	I S2 S3 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2	S4 S5 (RC (URM SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4	LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0	SCORE, S, (SW) C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1	(FD) 1.7 -0.9 -0.5 -0.5 -0.7 2.1 0.5 -0.1	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.1 Severe Vertical Irregularity, VL1 -1.2 -1.1 -1.0 -0.0 -0.1 -0.0 -0.0 -0.1 -0.1 0.0 -0.0 -0.1 -0.2 -0.1 -0.2 1.4 -0.2 1.4 -0.2 1.4 -0.2 1.4 Soil Type A or B 0.1 0.3 0.5 0.4	Image: Action of the system S2 S3 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA	S4 (RC (RC) S5 (URM (INF) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5	LEVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0	SCORE, S, (SW) C3 (URM (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4	(FD) 1.7 -0.9 -0.5 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5	-0.7 -0.4 -0.4 0.0 NA 0.3	1.5 NA NA -0.1 1.2 0.3
BASIC SCORE, N FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.' Severe Vertical Irregularity, V_{L1} -1.2 -1.1 -1.0 -0.0 -0.0 -0.1 0.1 0.0 -0.1 0.0 -0.2 1.4 Soil Type A or B 0.1 0.3 0.5 0.4 Soil Type E (> 3 stories) 0.2 0.2 0.1 -0.3 -0.6 -0.9 -0.3	AODIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 0 -1.0 -1.1 6 -0.6 -0.7 8 -0.7 -0.9 6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 2 -0.4 0.2 6 -0.6 NA 5 0.5 0.6	S4 S5 (RC (URM SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.6 -0.4 0.5 0.5	LEVEL 1 C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3	SCORE, S C2 C3 (SW) (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4	(FD) 1.7 -0.9 -0.5 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.7 Severe Vertical Irregularity, V_{L1} -1.2 -1.2 -1.2 -1.2 Moderate Vertical Irregularity, P_{L1} -0.7 -0.7 -0.7 -0.7 Plan Irregularity, P_{L1} -1.1 -1.0 -1.0 -0.0 Pre-Code -1.1 -1.0 -0.9 -0. Post-Benchmark 1.6 1.9 2.2 1.4 Soil Type A or B 0.1 0.3 0.5 0.4 Soil Type E (1-3 stories) 0.2 0.2 0.1 -0. Soil Type E (> 3 stories) -0.3 -0.6 -0.9 -0. Minimum Score, Smin 1.1 0.9 0.7 0.3 FINAL LEVEL 1 SCORE, SL1 ≥ Smin: 0.6; THEREFORE FEM	AODIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 0 -1.0 -1.1 6 -0.6 -0.7 8 -0.7 -0.9 6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 2 -0.4 0.2 6 -0.6 NA 5 0.5 0.6	S4 S5 (RC (URN, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5	LEVEL 1 C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~F	SCORE, S C2 C3 (SW) (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, M FEMA BUILDING TYPE Do Not Know W1 W1A W2 St Basic Score 3.6 3.2 2.9 2.' Severe Vertical Irregularity, V_{L1} -1.2 -1.1 -1.0 -0.0 -0.7 -0.7 -0.7 -0.7 -0.7 -0.7 -0.7 -0.7 -0.2 -0.1 -0.3 -0.5 0.4 0.3 0.5 0.4 0.3 0.5 0.4 -0.3 -0.6 -0.9 -0.2 0.1 -0.3 -0.6 -0.9 -0.3 -0.6 -0.9 <th>ADDIFIERS, A 1 S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 .4 1.4 1.1 .4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA .5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That That</th> <th>S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5</th> <th>LEVEL 1 C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~H ACTI</th> <th>SCORE, S C2 C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3</th> <th>L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2 RED</th> <th>(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3</th> <th>(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6</th> <th>-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2</th> <th>1.5 NA NA -0.1 1.2 0.3 -0.4 NA</th>	ADDIFIERS, A 1 S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 .4 1.4 1.1 .4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA .5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That That	S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5	LEVEL 1 C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~H ACTI	SCORE, S C2 C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3	L1 PC1 PC2 (TU) PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2 RED	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, MFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.'Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, V_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-0.9-0.Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.4Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, SMIN1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge SMIN$:0.6; THEREFORE FEINEXTENT OF REVIEWOTHExterior:PartialX All SidesDention:Dati NoneX visibleX EnteredDistailDati NoneX VisibleX Entered	ADDIFIERS, A 1 S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That Ied Structural Evalue	S4 S5 (RC (URN SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5	LEVEL 1 C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3	I SCORE, S, (SW) C2 (URM (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) CON REQUIF ed Structural Evence es, unknown FEM	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 i -0.5 i -0.7 i -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St (MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.9Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.4Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, S_{MIN} 1.10.90.70.4FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:0.6; THEREFORE FEREXTENT OF REVIEWExterior:PartialX All SidesA erialInterior:PartialX All SidesA erialDrawings Reviewed:X YesNoPartialPartial	ADDIFIERS, A 1 S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 .4 1.4 1.1 .4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA .5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That Ied Structural Evalue ounding potential (unitial structural st	S4 S5 (RC (URN SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 TAL IS ~H Detaile Yee Yee	SCORE, S C2 C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) ON REQUIF ed Structural Events es, core less tha	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 i -0.5 i -0.7 i -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.0Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.4Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, S_{MIN} 1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:0.6; THEREFORE FEINEXTENT OF REVIEWOTHDrawings Reviewed:YesNoSoil Type Source:NONEVisibleEnteredDrawings Reviewed:YesNoSCGeologic Hazards Source:DOGAMI-7-7	ADDIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 0 -1.0 -1.1 6 -0.6 -0.7 8 -0.7 -0.9 6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 2 -0.4 0.2 6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That Ied Structural Evalue ounding potential (uu uut-off, if known) alling hazards from the	S4 S5 (RC (URN SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 TAL IS ~H Detaile □ Yee Yee	SCORE, S C2 C3 (SW) (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.8 -0.5 -0.7 0.0 -0.2 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 0.3 HIGH (>10%) CON REQUIP CON REQUIP ed Structural Evences, score less that as, other hazards Stards <th>PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2</th> <th>(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 i -0.5 i -0.7 i -0.5 2.1 0.5 -0.1 -0.5 0.3</th> <th>(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3</th> <th>-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2</th> <th>1.5 NA NA -0.1 1.2 0.3 -0.4 NA</th>	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 i -0.5 i -0.7 i -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	-0.7 -0.4 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.0Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.4Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, SMIN1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge SMIN$:0.6; THEREFORE FERDEXTENT OF REVIEWOTHDrawings Reviewed:YesNoSoil Type Source:NONESibleEnteredDrawings Reviewed:YesNoSibleEnteredDrawings Reviewed:YesNoCitGeologic Hazards Source:DOGAMICitContact Person:JOSEPH GIPNERbit	ADDIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 0 -1.0 -1.1 6 -0.6 -0.7 8 -0.7 -0.9 6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 2 -0.4 0.2 6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That led Structural Evalu ounding potential (unut-off, if known) alling hazards from tuilding	S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5 PSE POTENT S Trigger A uation? nless SL2 > taller adjacent	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 TAL IS ~H Detaile □ Yee Yee □ Yee Yee	SCORE, S C2 C3 (SW) (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.8 -0.5 -0.7 0.0 -0.2 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 0.3 HIGH (>10%) CON REQUIP CON REQUIP ed Structural Evences, score less that as, other hazards Stards <th>PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2</th> <th>(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b</th> <th>(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding</th> <th>1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2</th> <th>1.5 NA NA -0.1 1.2 0.3 -0.4 NA</th>	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.0Pre-Code-1.1-1.0-1.0-0.9-0.7Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.4Soil Type E (1-3 stories)0.20.20.1-0.3Soil Type E (> 3 stories)-0.3-0.6-0.9-0.3Minimum Score, S_{MIN} 1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:0.6; THEREFORE FEIREXTENT OF REVIEWOTHDrawings Reviewed:XYesNoSoil Type Source:NONENOCOGeologic Hazards Source:DOGAMIFraContact Person:JOSEPH GIPNERDLEVEL 2 SCREENING PERFORMED?Si	ADDIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF Iter Hazards That led Structural Evalue ounding potential (unut-off, if known) alling hazards or Significant damage/de ignificant damage/de	ND FINAL I S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5 PSE POTENT S Trigger A uation? nless SL2 > taller adjacent Soil Type F	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~H Petaille □ Yee Yee □ Yee Yee ○ Detaille Yee ○ Detaille Yee	SCORE, S, (SW) S, (URM (URM INF) 2.0 C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) CON REQUIF es, unknown FEM es, other hazards other hazards other hazards	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b ecommen ed that shoc	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch puld be ev	1.0 -0.7 -0.4 0.0 NA 0.3 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.9Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.2Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, SMIN1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge SMIN$:0.6; THEREFORE FEIDEXTENT OF REVIEWOTHDrawings Reviewed:YesNoSoil Type Source:NONESerialProductGeologic Hazards Source:DOGAMIGGContact Person:JOSEPH GIPNERDDYes, Final Level 2 Score, S_{L2} XNoth	ADDIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF Iter Hazards That Ided Structural Evaluation Ided Structural Evaluation ounding potential (unut-off, if known) alling hazards from tuilding seologic hazards or S Iter Hazards or S	ND FINAL I S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5 PSE POTENT S Trigger A uation? nless SL2 > taller adjacent Soil Type F	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~F Petaile □ Yee Yee □ Detaile Yee □ Yee Not Detaile Yee □ Yee Not	I SCORE, S, (SW) C2 (SW) C3 (URM (NF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) CON REQUIF es, unknown FEM o, onstructural function o, onstructural h	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b ecommen ed that sho at may requ	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch puld be ev	1.0 -0.7 -0.4 0.0 NA 0.3 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.0Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.2Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, SMIN1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge SMIN$:0.6; THEREFORE FEIEXTENT OF REVIEWOTHDrawings Reviewed:YesNoSoil Type Source:NONESerialPrectalDrawings Reviewed:YesNoSerialPrectalDrawings Reviewed:YesNoSerialPrectalContact Person:JOSEPH GIPNERDGAMIFaLEVEL 2 SCREENING PERFORMED?SerialSerial	ADDIFIERS, A S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 -0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF Iter Hazards That led Structural Evalue ounding potential (unut-off, if known) alling hazards or Significant damage/de ignificant damage/de	ND FINAL I S4 S5 (RC (URM, SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5 PSE POTENT S Trigger A uation? nless SL2 > taller adjacent Soil Type F	LEVEL 1 C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 TAL IS ~H Detaile Ye Ye Ottaile Ye Ottaile Ye Xe Ottaile Ye Detaile Ye Xe Ottaile Ye Xe Ye Xe Ye Ye Ye Ye Ye Ye Ye Ye Ye	SCORE, S, (SW) S, (URM (URM INF) 2.0 C3 (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) CON REQUIF es, unknown FEM es, other hazards other hazards other hazards	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b ecommen ed that sho at may requ y	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch puld be ev	1.0 -0.7 -0.4 0.0 NA 0.3 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
BASIC SCORE, NFEMA BUILDING TYPEDo Not KnowW1W1AW2St MRBasic Score3.63.22.92.7Severe Vertical Irregularity, V_{L1} -1.2-1.2-1.2-1.2Moderate Vertical Irregularity, P_{L1} -0.7-0.7-0.7-0.7Plan Irregularity, P_{L1} -1.1-1.0-1.0-0.9Pre-Code-1.1-1.0-0.9-0.Post-Benchmark1.61.92.21.4Soil Type A or B0.10.30.50.2Soil Type E (1-3 stories)0.20.20.1-0.Soil Type E (> 3 stories)-0.3-0.6-0.9-0.Minimum Score, SMIN1.10.90.70.3FINAL LEVEL 1 SCORE, $S_{L1} \ge SMIN$:0.6; THEREFORE FEIDEXTENT OF REVIEWOTHDrawings Reviewed:YesNoSoil Type Source:NONESerialProductGeologic Hazards Source:DOGAMIGGContact Person:JOSEPH GIPNERDDYes, Final Level 2 Score, S_{L2} XNoth	ADDIFIERS, A 1 S2 S3 (BR) (LM) 1 2.0 2.6 .0 -1.0 -1.1 .6 -0.6 -0.7 .8 -0.7 -0.9 .6 -0.6 0.8 4 1.4 1.1 4 0.6 0.1 .2 -0.4 0.2 .6 -0.6 NA 5 0.5 0.6 MA 154 COLLAF IER HAZARDS here Hazards That Ied Structural Evalue ounding potential (unut-off, if known) alling hazards from tuilding seologic hazards from tuilding seologic hazards or S ignificant damage/de he structural system	ND FINAL S4 S5 (RC (URN SW) 2.0 1.7 -1.0 -0.8 -0.6 -0.5 -0.7 -0.6 -0.6 -0.2 1.9 NA 0.6 0.5 -0.1 -0.4 -0.5 0.5 PSE POTENT S Trigger A uation? nless $S_{L2} >$ taller adjacent Soil Type F eterioration to to	LEVEL 1 C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 IAL IS ~-F Petaille □ Yee Yee □ Detaille Yee □ Yee Not □ Detaille Yee □ Yee Not □ Not Not □ Not Not	SCORE, S, (SW) S, (URM (URM INF) 2.0 1.2 -1.0 -0.7 -0.6 -0.4 -0.8 -0.5 -0.7 -0.1 2.1 NA 0.5 0.3 0.0 -0.2 -0.7 -0.3 0.3 0.3 HIGH (>10%) PON REQUIF es, unknown FEM us, score less that us, other hazards on on on on	PC1 (TU) PC2 PC2 1.6 1.4 -1.0 -0.9 -0.6 -0.5 -0.7 -0.6 -0.5 -0.3 2.0 2.4 0.6 0.4 -0.3 -0.1 NA -0.4 0.2 0.2	(FD) 1.7 -0.9 -0.5 -0.7 2.1 0.5 -0.1 -0.5 0.3 ired? e or other b ecommen ed that shc at may requ ry tiffed	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 uilding ded? (ch build be ev uire mitig	1.0 -0.7 -0.4 0.0 NA 0.3 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0

Rapid Visual Screening of Buildings for Potential Seismic Hazards

FEMA P-154 Data Collection Form

					Add	ress:	209 N	W 10th	Street							
							Redmo	ond, Ol	R			Z	ip:	97756		
					Oth	er Identi	ifiers:									
							me: De	esc_scl	h08D							
						: Scho										
	227				1000	-	44.2778	38		L	ongitu	de: -1	21.179	88		
State of the state	CONSTRUCTION OF	-	N. M		Ss:						S1:					
	-			1		eener(s)	-	C / JAC				ate/Time			er 2021	
					and the second se		: Abov			Belov	v Grade	: 0			1964 🛛	
	- /-				and the second se		Area (so		6,150	00r(-) P	.:14.		Code	Year:	1961 L	JBC
	and and	-				litions:	X N] Yes, Y				—	-4'		
	ALL			-	Occ	upancy	-	embly Istrial Iy	Commer Office Warehou	[Emer. So School Residen	tial, #Un	_	storic overnmei	☐ Sheltent	er
	-		-	and a state	Soil	Туре:	□A Hard Rock	□B Avg Rock	Dens Soil	e St	iff S	oft Po		N K DNK, ass	ume Type	D.
A STATE		14	1.	0	Geo	logic Ha	azards:	Liquefac	ction: Yes	/No/DNk	Lands	lide: Yes	No/DNK	Surf. R	upt.: Yes	Jo DNK
Dese se	108	1238	110	Page-	_	acency:			ounding						t Building	
M.C.		-	Re-	2	_	gularitie			ertical (typ an (type)	oe/severi	ty)					
and the second second			-		Exte	erior Fal	lina		nbraced (himnev		V Hea	wy Clade	lina or H	leavy Ven	eer
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Ċ.	BA	20	The state			MMENT	C .	0	tner:							
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	and the second	1	-		H											
Birch Ave			-		H											
- 1 - 1 - 1 - 1	W.Lat	111	24	- T												
			N													
SKE	тсн					Addition	al sketche	es or cor	nments o	n separa	ite page					
			RE, MC	DIFIE							1.0					
FEMA BUILDING TYPE Do Not		V1A W2	S1	\$2	\$3	S4	S5	C1	C2	C3	PC1	PC2	RM1	RM2	URM	МН
Know			(MRF)	(BR)	(LM)	(RC SW)	(URM INF)	(MRF)	(SW)	(URM INF)	(TU)		(FD)	(RD)		
Basic Score	3.6 3	3.2 2.9	2.1	2.0	2.6	2.0	1.7	1.5	2.0	1.2	1.6	1.4	1.7	1.7	1.0	1.5
Severe Vertical Irregularity, VL1		1.2 -1.2		-1.0	-1.1	-1.0	-0.8	-0.9	-1.0	-0.7	-1.0	-0.9	-0.9	-0.9	-0.7	NA
Moderate Vertical Irregularity, V_{L1}		0.7 -0.7		-0.6	-0.7	-0.6	-0.5	-0.5	-0.6	-0.4	-0.6	-0.5	-0.5	-0.5	-0.4	NA
Plan Irregularity, <i>P</i> _{L1} Pre-Code		1.0 -1.0 1.0 -0.9		-0.7 -0.6	-0.9 -0.8	-0.7 -0.6	-0.6 -0.2	-0.6 -0.4	-0.8 -0.7	-0.5 -0.1	-0.7 -0.5	-0.6 -0.3	-0.7 -0.5	-0.7 -0.5	-0.4 0.0	NA -0.1
Post-Benchmark		1.9 2.2		-0.0	-0.0	-0.0	-0.2 NA	-0.4 1.9	-0.7	-0.1 NA	-0.5 2.0	-0.3 2.4	-0.5	-0.5	NA	-0.1 1.2
Soil Type A or B	· · ·	0.3 0.5		0.6	0.1	0.6	0.5	0.4	0.5	0.3	0.6	0.4	0.5	0.5	0.3	0.3
Soil Type E (1-3 stories)		0.2 0.1		-0.4	0.2	-0.1	-0.4	0.0	0.0	-0.2	-0.3	-0.1	-0.1	-0.1	-0.2	-0.4
Soil Type E (> 3 stories)		0.6 -0.9		-0.6	NA	-0.6	-0.4	-0.5	-0.7	-0.3	NA	-0.4	-0.5	-0.6	-0.2	NA
Minimum Score, S _{MIN}		0.9 0.7		0.5	0.6	0.5	0.5	0.3	0.3	0.3	0.2	0.2	0.3	0.3	0.2	1.0
FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:	1.0; TH	IEREFO	RE FEMA	A 154 C	OLLAP	SE PO	TENTIA	AL IS ~I	HIGH (1	0%)						
EXTENT OF REVIEW			OTHE	r haz	ARDS	i		АСТ	ION RI	EQUIR	RED					
	ll Sides 🔀		Are The				4	Detail	ed Struct	ural Eva	aluation	Require	d?			
Interior: Interior: None X Vi Drawings Reviewed: X Yes N		Entered	Detailed						es, unkno			ng type or	other bu	uilding		
Soil Type Source: NONE	0		Pour	nding pot off, if kno		nless SL2	>		es, score es, other l							
Geologic Hazards Source: DOGAMI						aller adja	icent			iazaius	hiesellí					
			build	ling						ructural	Evaluat	tion Rec	ommen	led? (ch	neck one)	
Contact Person: JOSEPH GIPNE	_1\			lania ha-	ards or S	ioil Type	F							,	'	
				logic naz		ion Type	'.		as nonetr	uctural h	lazarde i	dentified	that cho		hoteulev	
LEVEL 2 SCREENING PERFO	RMED?		🗌 Sign	ificant da	image/de	terioratio	on to							uld be e iire mitig		а
LEVEL 2 SCREENING PERFO Yes, Final Level 2 Score, SL2	RMED?	X No	🗌 Sign	ificant da structural	image/de	terioratio	on to	X No de	o, nonstru tailed eva	ictural ha aluation i	azards e s not ne	xist that r cessary	nay requ	iire mitig	valuated ation, but	а
LEVEL 2 SCREENING PERFO	RMED?		🗌 Sign	ificant da	image/de	terioratic	on to	X No de	o, nonstru	ictural ha aluation i	azards e s not ne	xist that r cessary	nay requ			а
LEVEL 2 SCREENING PERFO Yes, Final Level 2 Score, SL2	RMED?	X No No erified, sc	Sign the s	ificant da structural	image/de system he follow	iterioratio	on to	⊠ No de ⊡ No mated o	o, nonstru tailed eva o, no non r unrelia	ictural ha aluation i structura ble data	azards e s not ne l hazard <u>OR</u> l	xist that r cessary s identifie	may requ ed [o Not Kr	iire mitig		a

Rapid Visual Screening of Buildings for Potential Seismic Hazards

FEMA P-154 Data Collection Form

							Add	ress:	209 N	W 10th	Street							
							1		Redmo	ond, OF	२			Z	Zip:	97756		
A AND		4	-	P	1	A	Othe	er Identi	ifiers:									
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A MANAR		100100	TT	-	and the second s	1		: Scho										
BARRIER MARKET A	Sector Party in the	TIT	E			***		-	44.2778	38			Longitu	de: -1	21.179	88		
						16 m	Ss:						Longitu S₁:	ue	121.175	00		
	a.		T			1		eener(s)	: <u>SL</u>	.C / JAC	3			ate/Tim	e: No	vembe	r 2021	
										e Grade			w Grade	: 0	_		1990 🕻	X EST
			and the			And the second		al Floor . litions:	Area (so		3,200 Yes, Y		Built:		_ Code	Year:		
THE REAL PROPERTY OF A DESCRIPTION OF A	-							upancy		embly	Comme		Emer. S	ervices		storic	□ Shelt	er
A States					1000					istrial ty	Office Wareho	use	School Residen	itial, #Ur	_	overnmer		
South and	1	-	-	12		-	Soil	Туре:	□A Hard Rock	□B Avg Rock	Den: Soi	se S	tiff S	oft P	-	N K DNK, assi	ume Type	D.
195 A 5				3			Geo	logic Ha	azards:	Liquefac	tion: Yes	/No/DN	K Lands	lide: Yes	/No/DNK	Surf. Ru	upt.: Yesi	No/DNK
Total Total	SC SC	h08		1	and a	and the second	Adja	acency:		D Pc	ounding		Falling H	azards fr	om Taller	Adjacen	t Building	
	and the second			14	WHIN		Irrec	gularitie	s:	🗌 Ve	ertical (ty	pe/sever	rity)					
The second second				-		- 22	H	-			an (type)			RANT	CORNE	S, L-SI	HAPE	
A CONTRACTOR OF STREET	1.00				-	350	Exte	erior Fal	lina	U II	nbraced	Chimne	/s	X Hea	avy Cladd	lina or H	eavv Ver	neer
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6	100	+	11. 1	0						0	ther:							
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and the state	Logic Die	-	areas	*	ACT DIA S	62.8	H											
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FEMA BUILDING TYPE	SKE Do Not Know	B/ W1	W1A	W2	S1 (MRF)	S2 (BR)	RS, AI S3 (LM)	ND FIN S4 (RC SW)	S5 (URM INF)	C1 (MRF)	1 SCO C2 (SW)	RE, S C3 (URM INF)	PC1 (TU)	PC2	RM1 (FD)	RM2 (RD)	URM	МН
Basic Score	Do Not	B/ W1 3.6	W1A 3.2	W2 2.9	S1 (MRF) 2.1	S2 (BR) 2.0	RS, Al S3 (LM) 2.6	ND FIN (RC SW) 2.0	S5 (URM INF) 1.7	EVEL 1 (MRF) 1.5	1 SCO C2 (SW) 2.0	RE, S C3 (URM INF) 1.2	L1 PC1 (TU) 1.6	PC2	(FD) 1.7	(RD) 1.7	1.0	1.5
Basic Score Severe Vertical Irregularity, VL1	Do Not	B/ W1 3.6 -1.2	W1A 3.2 -1.2	W2 2.9 -1.2	S1 (MRF) 2.1 -1.0	S2 (BR) 2.0 -1.0	RS, AI S3 (LM) 2.6 -1.1	ND FIN S4 (RC SW) 2.0 -1.0	S5 (URM INF) 1.7 -0.8	C1 (MRF) 1.5 -0.9	1 SCO (SW) 2.0 -1.0	RE, S C3 (URM INF) 1.2 -0.7	L1 PC1 (TU) 1.6 -1.0	PC2 1.4 -0.9	(FD) 1.7 -0.9	(RD) 1.7 -0.9	1.0 -0.7	1.5 NA
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1}	Do Not	B/ W1 3.6 -1.2 -0.7	W1A 3.2 -1.2 -0.7	W2 2.9 -1.2 -0.7	S1 (MRF) 2.1 -1.0 -0.6	S2 (BR) 2.0 -1.0 -0.6	RS, AI S3 (LM) 2.6 -1.1 -0.7	ND FIN (RC SW) 2.0 -1.0 -0.6	S5 (URM INF) 1.7 -0.8 -0.5	C1 (MRF) 1.5 -0.9 -0.5	1 SCO (SW) 2.0 -1.0 -0.6	RE, S (URM INF) 1.2 -0.7 -0.4	L1 PC1 (TU) 1.6 -1.0 -0.6	PC2 1.4 -0.9 -0.5	(FD) 1.7 -0.9 -0.5	(RD) 1.7 -0.9 -0.5	1.0 -0.7 -0.4	1.5 NA NA
Basic ScoreSevere Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1}	Do Not	B/ W1 3.6 -1.2 -0.7 -1.1	W1A 3.2 -1.2 -0.7 -1.0	W2 2.9 -1.2 -0.7 -1.0	S1 (MRF) 2.1 -1.0 -0.6 -0.8	S2 (BR) -1.0 -0.6 -0.7	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7	S5 (URM INF) 1.7 -0.8 -0.5 -0.6	C1 (MRF) 1.5 -0.9 -0.5 -0.6	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8	RE, S C3 (URM INF) 1.2 -0.7 -0.4 -0.5	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7	PC2 1.4 -0.9 -0.5 -0.6	(FD) 1.7 -0.9 -0.5 -0.7	(RD) 1.7 -0.9 -0.5 -0.7	1.0 -0.7 -0.4 -0.4	1.5 NA NA NA
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-Code	Do Not	B/ W1 3.6 -1.2 -0.7	W1A 3.2 -1.2 -0.7	W2 2.9 -1.2 -0.7	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6	S2 (BR) 2.0 -1.0 -0.6	RS, AI S3 (LM) 2.6 -1.1 -0.7	ND FIN (RC SW) 2.0 -1.0 -0.6	S5 (URM INF) 1.7 -0.8 -0.5	C1 (MRF) 1.5 -0.9 -0.5	1 SCO (SW) 2.0 -1.0 -0.6	RE, S (URM INF) 1.2 -0.7 -0.4	L1 PC1 (TU) 1.6 -1.0 -0.6	PC2 1.4 -0.9 -0.5	(FD) 1.7 -0.9 -0.5	(RD) 1.7 -0.9 -0.5	1.0 -0.7 -0.4	1.5 NA NA
Basic ScoreSevere Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1}	Do Not	B / 3.6 -1.2 -0.7 -1.1 -1.1	W1A 3.2 -1.2 -0.7 -1.0 -1.0	W2 2.9 -1.2 -0.7 -1.0 -0.9	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4	S2 (BR) -1.0 -0.6 -0.7 -0.6	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8	ND FIN (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7	RE, S C3 (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5	PC2 1.4 -0.9 -0.5 -0.6 -0.3	(FD) 1.7 -0.9 -0.5 -0.7 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5	1.0 -0.7 -0.4 -0.4 0.0	1.5 NA NA NA -0.1
Basic Score Severe Vertical Irregularity, V _{L1} Moderate Vertical Irregularity, V _{L1} Plan Irregularity, P _{L1} Pre-Code Post-Benchmark	Do Not	B / W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9	W2 -1.2 -0.7 -1.0 -0.9 2.2	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1	S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9	1 SCO (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4	(FD) -0.9 -0.5 -0.7 -0.5 2.1	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1	1.0 -0.7 -0.4 -0.4 0.0 NA	1.5 NA NA -0.1 1.2
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)	Do Not	B 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA	S4 (RC SW) 2.0 -1.0 -0.6 1.9 0.6 -0.1	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5	1 SCO (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic ScoreSevere Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-CodePost-BenchmarkSoil Type A or BSoil Type E (1-3 stories)	Do Not	B , w1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2	W2 2.9 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2	ND FIN (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0	1 SCO (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)	Do Not Know	B , W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 <i>1.1</i>	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6	S2 (BR) 2.0 -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5	RS, AI 33 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6	ND FIN S4 (RC SW) -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 -0.4 0.5	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3	1 SCO (SW) 2.0 -1.0 -0.6 -0.7 2.1 0.5 0.0 -0.7 0.3	RE, S (URM INF) -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic ScoreSevere Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L1} Plan Irregularity, P_{L1} Pre-CodePost-BenchmarkSoil Type A or BSoil Type E (1-3 stories)Soil Type E (> 3 stories)Minimum Score, S_{MIN}	Do Not Know	B , W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 <i>1.1</i>	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 0.2 -0.6 0.5	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 C0	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 -0.4 0.5	EVEL 1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~H	1 SCO (SW) 2.0 -1.0 -0.6 -0.7 2.1 0.5 0.0 -0.7 0.3	RE, S C3 (UR INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%)	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti	Do Not Know	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0;	W1A 3.2 -1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.2 -0.6 0.5 RE FEMA OTHEL Are Ther	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.4 -0.6 0.5 . 154 CU R HAZ e Hazard	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CLAP ds That T	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger 4	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	C1 (MRF) -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~H	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (*	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF	L1 PC1 (TU) -1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 -0.4 -0.1 -0.4 0.2	(FD) 1.7 -0.9 -0.5 0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None	Do Not Know	B , W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0; VII Sides fisible	W1A 3.2 -1.2 -0.7 -1.0 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 -0.2 -0.6 0.5 RE FEMA OTHEI Are Ther Detailed	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 . 154 C0 R HAZ e Hazaro Structur	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation?	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~H Detaile □ Ye	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (⁷ ION R ed Struc es, unkno	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pown FEM	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require	(FD) 1.7 -0.9 -0.5 0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes	Do Not Know	B , W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0; VII Sides fisible	W1A 3.2 -1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 -0.2 -0.6 0.5 RE FEMA OTHEI Are Ther Detailed □ Pour	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 .154 C0 R HAZ e Hazaro Structur ding pote	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A ation?	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	EVEL 1 C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~H Detaile □ Ye □ Ye	I SCO C2 (SW) 2.0 -1.0 -0.6 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R ed Structor es, score	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pown FEM less tha	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir n cut-off	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed?	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE	Do Not Know	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0; VII Sides Visible Jo	W1A 3.2 -1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.5 RE FEMA OTHEL Are Ther Detailed □ Pour cut-cut-cut-cut-cut-cut-cut-cut-cut-cut-	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.4 -0.6 0.5 . 154 CU R HAZ e Hazard Structur ding pote ff, if know	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn)	S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A lation?	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~F Detaile □ Yee □ Yee □ Yee	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (* ION R ed Struct ss, score ss, other	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pown FEM less tha	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir n cut-off	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed?	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source:	Do Not Know	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0; VII Sides Visible Io	W1A 3.2 -1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 1.4 0.4 -0.5 RE FEMA OTHEL Are Ther Detailed □ Pour cut-cut-cut-cut-cut-cut-cut-cut-cut-cut-	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.4 -0.6 0.5 . 154 Cu R HAZ e Hazaro Structur ding pote ff, if know g hazaro	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un	S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO Frigger A lation?	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.4 0.5 TENTIA	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~-F Detaile □ Yee □ Yee □ Yee	I SCO C2 (SW) 2.0 -1.0 -0.6 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (* ION R ed Struc es, score es, other	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pwn FEN less tha hazards	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir n cut-off present	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed?	(RD) 1.7 -0.9 -0.5 2.1 0.5 -0.1 -0.6 0.3 wilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF	Do Not Know	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 7.1 1.0; VII Sides lo L ER	W1A 3.2 -1.2 -0.7 -1.0 -1.0 0.3 0.2 -0.6 0.9 THERI X Aerr X Ent	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 -0.2 -0.6 0.5 RE FEMA OTHEI Are Ther Detailed □ Pourc □ Fallin build □ Geol	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 .154 C0 R HAZ e Hazaro Structur ding pote ff, if know ig hazaro ogic haza	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn) ds from ta ards or S	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO SE PO Frigger A nation? nless SL2 aller adja	S5 OUT INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.5 TENTIA -0.5	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.55 0.3 AL IS ~-H Detaile □ Ye □ Ye □ Ye □ Ye □ Detaile	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R ed Struc es, unknows, score s, other other and the structure of the	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pown FEM less tha hazards tructura	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED raluation 1A buildir n cut-off present I Evalua	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? r other bu	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 wilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source:	Do Not Know	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 7.1 1.0; VII Sides lo L ER	W1A 3.2 -1.2 -0.7 -1.0 -1.0 0.3 0.2 -0.6 0.9 THERI X Aerr X Ent	W2 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 0.2 -0.6 0.5 RE FEMA Are Ther Detailed Pour cut-cut-cut-cut-cut-cut-cut-cut-cut-cut-	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 .154 C0 R HAZ e Hazaro Structur ding pote ff, if know og hazaro ing ogic haza ficant da	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn) ds from ta ards or S mage/de	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO SE PO Frigger A nation? nless SL2 aller adja	S5 OUT INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.5 TENTIA -0.5	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~-F Detaile □ Yee □ Yee □ Yee □ Yee □ Yee □ Yee	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R ed Struc es, score es, other o de Nons es, nonst	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev pwn FEN less tha hazards tructural	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation 1A buildir n cut-off present I Evalua hazards	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? or other but commence d that show	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 wilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: JOSEF	Do Not Know 1 ≥ Smin: al X A al X A al X N DOGAM PH GIPNI PERFC	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 7.1 1.0; VII Sides lo L ER	W1A 3.2 -1.2 -0.7 -1.0 -1.0 0.3 0.2 -0.6 0.9 THERI X Aerr X Ent	w2 2.9 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR EFOR	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 0.2 -0.6 0.5 RE FEMA Are Ther Detailed Pour cut-cut-cut-cut-cut-cut-cut-cut-cut-cut-	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 .154 C0 R HAZ e Hazaro Structur ding pote ff, if know ig hazaro ogic haza	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn) ds from ta ards or S mage/de	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO SE PO Frigger A nation? nless SL2 aller adja	S5 OUT INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.5 TENTIA -0.5	EVEL 1 C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~H Detaile □ Ye	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R ed Struc es, score es, other o de Nons es, nonst	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev bown FEM less that hazards tructural ructural h	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir n cut-off present I Evalua hazards azards e	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec identified xist that	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? r other bu	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 wilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: □ Parti Interior: □ None Drawings Reviewed: □ Yes Soil Type Source: NONE Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING □ Yes, Final Level 2 Score, SL	Do Not Know 1 ≥ Smin: al X A al X A al X N DOGAM PH GIPNI PERFC	B/ W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 7.1 1.0; VII Sides lo L ER	W1A 3.2 -1.2 -0.7 -1.0 1.9 0.3 0.2 -0.6 0.9 THERI X Aerr X Ent	w2 2.9 -1.2 -0.7 -1.0 -0.9 2.2 0.5 0.1 -0.9 0.7 EFOR ial ered	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 0.2 -0.6 0.5 RE FEMA Are Ther Detailed Pour cut-cut-cut-cut-cut-cut-cut-cut-cut-cut-	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 .154 C0 R HAZ e Hazaro Structur ding pote ff, if know og hazaro ing ogic haza ficant da	RS, AI S3 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn) ds from ta ards or S mage/de	ND FIN S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 -0.1 -0.6 0.5 SE PO SE PO Frigger A nation? nless SL2 aller adja	S5 OUT INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.4 -0.5 -0.5 TENTIA -0.5	VEL 1 C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~F Detaild □ Ye □ Ye X Nc Detaild □ Ye X Nc Detaild □ Ye X Nc de	I SCO C2 (SW) 2.0 -1.0 -0.6 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R ed Struct es, score es, score es, score es, nonstruct p, nonstruct	RE, S (UR (UR INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIE tural Ev bown FEM less that hazards tructural in uctural in aluation	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation IA buildir n cut-off present I Evalua hazards azards e is not ne	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec xist that cessary	(FD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? or other but commence d that show may required	(RD) 1.7 -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.6 0.3 wilding	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, SL EXTENT OF REVIEW Exterior: Parti Interior: None Drawings Reviewed: Yes Soil Type Source: NONE Geologic Hazards Source: Coster Contact Person: JOSEF LEVEL 2 SCREENING Yes, Final Level 2 Score, SL Nonstructural hazards?	Do Not Know 1 ≥ Smin: al X A al X A by V DOGAM PH GIPNI PERFC 2 Yes	B, W1 3.6 -1.2 -0.7 -1.1 -1.1 1.6 0.1 0.2 -0.3 1.1 1.0; UI Sides Visible Jo RMEI	W1A 3.2 -1.2 -0.7 -1.0 -1.0 0.3 0.2 -0.6 0.9 THERI X Aerr X Ent	w2 2.9 -1.2 -0.7 -1.0.9 2.2 0.5 0.1 -0.9 0.7 EFOR ial ered lo lo	S1 (MRF) 2.1 -1.0 -0.6 -0.8 -0.6 0.4 -0.2 -0.6 0.5 RE FEMA OTHEI Are Ther Detailed Pour cut-c Fallir build Geol Sign the s	S2 (BR) -1.0 -0.6 -0.7 -0.6 1.4 0.6 -0.4 -0.6 0.5 154 C0 R HAZ e Hazaro Structur ding pote ff, if knov ug hazaro ing ogic haza fricant da tructural	RS, AI RS, AI 33 (LM) 2.6 -1.1 -0.7 -0.9 -0.8 1.1 0.1 0.2 NA 0.6 OLLAP CARDS ds That T ral Evalu ential (un wn) ds from ta ards or S mage/de system	S4 (RC SW) 2.0 -1.0 -0.6 -0.7 -0.6 1.9 0.6 0.05 SE PO 5 Frigger A ation? aller adja ioil Type distribute	S5 (URM INF) 1.7 -0.8 -0.5 -0.6 -0.2 NA 0.5 -0.4 -0.5 -0.4 -0.5 cent F n to	C1 (MRF) 1.5 -0.9 -0.5 -0.6 -0.4 1.9 0.4 0.0 -0.5 0.3 AL IS ~-F Detaile Ye No Detaile Ye No Detaile Ye No Octaile No Octaile No Octaile No No No	1 SCO C2 (SW) 2.0 -1.0 -0.6 -0.8 -0.7 2.1 0.5 0.0 -0.7 0.3 HIGH (7 ION R id Struct as, unknown s, score as, onstruct b, nonstruct tailed ev p, no non	RE, S (URM INF) 1.2 -0.7 -0.4 -0.5 -0.1 NA 0.3 -0.2 -0.3 0.3 10%) EQUIF tural Ev bown FEM less tha hazards tructural h aluation astructural	L1 PC1 (TU) 1.6 -1.0 -0.6 -0.7 -0.5 2.0 0.6 -0.3 NA 0.2 RED aluation A buildir n cut-off present I Evalua hazards azards e is not ne al hazard	PC2 1.4 -0.9 -0.5 -0.6 -0.3 2.4 0.4 -0.1 -0.4 0.2 Require ng type o tion Rec identified xist that cessary Is identified	(FD) -0.9 -0.5 -0.7 -0.5 2.1 0.5 -0.1 -0.5 0.3 ed? or other but commence it that show may required	(RD) 1.7 -0.9 -0.5 2.1 0.5 -0.1 -0.6 0.3 iilding iilding iild be ev ire mitiga DNK	1.0 -0.7 -0.4 -0.4 0.0 NA 0.3 -0.2 -0.2 0.2 0.2	1.5 NA NA -0.1 1.2 0.3 -0.4 NA 1.0
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Seismic Evaluation Report For:

TUMALO ELEMENTARY SCHOOL

19835 2nd St, Tumalo, OR 97703 Redmond School District

Prepared By: ZCS Engineering & Architecture Matthew R. Smith, PE, SE, Principal 524 Main Street, Suite 2, Oregon City, OR 97045 T: 503.659.2205 | E: MattS@zcsea.com





Building Part	Building Part Name	Included in Retrofit	Year Built	Building Type***	Nonstructural Retrofits Included in Scope Y/N***	Previous Seismic Retrofit Y/N*** (Year if Yes)
А	Classroom	Ν	1918			
В	Classroom	Ν	1930			
С	Classroom	Ν	1958			
D	Classroom	Υ	1950	URM	Y	N
E	Gymnasium	Υ	1958	URM	Y	N
F	Classroom	Ν	1970			
G	Classroom	Ν	1994			
Н	Classroom	Y	1986	URM	Y	Ν
Seismic fra	agility inputs for ex	kisting buildin	gs with I	previous seis	smic retrofits MUS	T be adjusted to
reflect pre	evious seismic retr	ofit measures	comple			T be adjusted to
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reflect pre Total Retro Retrofit Sc Retrofit Co	evious seismic retr ofit Cost quare Feet ost per	ofit measures \$2,481,875 15,600	comple			T be adjusted to
reflect pre Total Retro Retrofit So Retrofit Co Square Fo	evious seismic retr ofit Cost quare Feet ost per	ofit measures \$2,481,875 15,600 \$159.09	s comple	ted for a bu	ilding part.	T be adjusted to

Engineer	ing Report Checklist	
\boxtimes	Engineering Report Cover Page	
\boxtimes	Project Summary Page	Page 1
\boxtimes	Building Parts Identification	Page 5
\boxtimes	Statement of the Performance Objective	Page 7
	Summary of Deficiencies	
\boxtimes	Structural Seismic Deficiencies	Page 11
\boxtimes	Nonstructural Seismic Deficiencies	Page 12
	Summary of Mitigation/Retrofit	
\boxtimes	Structural Mitigation/Retrofit	Page 11
\boxtimes	Nonstructural Mitigation/Retrofit	Page 12
	Summary Construction Cost Estimate	
\boxtimes	Direct Cost	Page 15
\boxtimes	Indirect Soft Cost	Page 15
\boxtimes	Certification Statement by Engineer	Page 16
	ASCE 41-17 Tier 1 Checklist	
\boxtimes	Basic Configuration Checklist	Appendix B
\boxtimes	Building System Structural Checklist	Appendix B
\boxtimes	Nonstructural Checklist	Appendix B
X	Retrofit Drawings & Sketches	Appendix C
\boxtimes	DOGAMI or Geotechnical Report	Appendix D
\boxtimes	Itemized Construction Cost Estimate	Appendix E
\boxtimes	Rapid Visual Screening	Appendix F

1.0 Project Introduction

Redmond School District is located in Redmond, Oregon in Deschutes County. The District operates ten schools located within the community including the property of interest, Tumalo Elementary School. The District has retained ZCS Engineering and Architecture (ZCS) to perform a seismic evaluation of Tumalo Elementary School that provides the District with an objective, comprehensive analysis of the condition of the building's seismic resisting systems. The purpose of the evaluation is to determine the seismic lateral resisting system deficiencies when compared to buildings designed using modern building codes. This evaluation was performed in accordance with the American Society of Civil Engineers "Seismic Rehabilitation of Existing Buildings ASCE/SEI 41-17".

SEISMIC EVALU	ATION SNAPSHOT
Street Address	19835 2 nd Street, Tumalo, OR 97703
Evaluation Standard	ASCE 41-17 (Tier 1 Analysis)
Target Building Performance Level	Immediate Occupancy – BSE-1E; Life Safety – BSE-2E
Target Non-Structural Performance Level	Position Retention – BSE-1E; Hazard Reduced – BSE-2E
ASCE 41 Building Type	URM
Site Soil Classification	D
Seismic Zone Hazard Level	Moderately High
Cost Estimate	\$2,481,875

2.0 Building Description

The Gymnasium area 'E' was constructed in 1958 with an approximate footprint 9,000-square-feet. The gymnasium roof consists of 2x4 laminated deck over arched glulam beams with perimeter and interior under-reinforced masonry walls. The exterior masonry walls of the gymnasium are partial height with wood framed walls above. This structure has been classified as URM due to the lack of adequate reinforcement. Foundations consist of slab-on-grade with continuous reinforced concrete footings.

The classroom addition area 'H' was constructed in 1986 with an approximate footprint of 4,000-squarefeet. The roof consists of wood trusses with plywood sheathing supported by original under-reinforced masonry walls that were altered during the addition and wood framed walls supporting the roof of area 'H' and the adjacent hallway. The building has been classified as a URM. URM walls are present on two sides and non-compliant wood walls on the other two sides. Foundations consist of slab-on-grade with continuous reinforced concrete footings.

Classroom area 'D' was constructed in the 1950s with an approximate footprint of 2,600-square-feet. The roof consists of wood trusses with straight sheathed roof diaphragm supported by under reinforced masonry walls. This structure has been classified as URM due to the lack of adequate reinforcement. Foundations consist of slab-on-grade with continuous reinforced concrete footings.

Photographs of the building parts included in this report are located in Appendix A.

Project No: P-2706-21

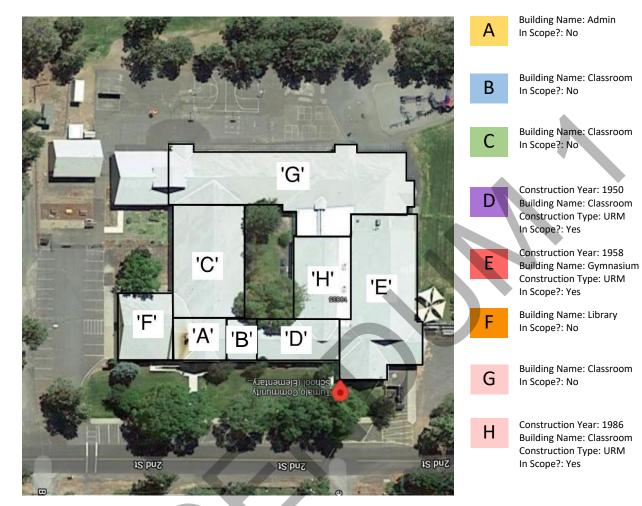


Figure 1

Tumalo Community School Key Plan

3.0 Definition of Building Types

After reviewing the facility and the existing drawings we have determined the lateral system is defined as URM. Per ASCE 41-17 the subject structure's lateral system is defined as:

Unreinforced Masonry Bearing Walls URM – This building was initially reviewed as an RM1 construction type due to the presence of some reinforcing present in the wall construction. Through the RM1 Tier 1 evaluation it was determined that the walls are under reinforced. Accordingly, this building is classified as a URM. These buildings have a perimeter bearing walls that consist of unreinforced clay brick, stone, or concrete masonry. Interior bearing walls, where present, also consist of unreinforced clay brick, stone, or concrete masonry. In older construction, floor and roof framing consists of straight or diagonal lumber sheathing supported by wood joists, which, in turn, are supported on posts and timbers. In more recent construction, floors consist of structural panel or plywood sheathing rather than lumber sheathing. The diaphragms are flexible relative to the walls. Where they exist, ties between the walls and the diaphragms consist of anchors or bent steel plates embedded in the mortar joints and attached to framing. The foundation system may consist of a variety of elements.

4.0 Seismic Evaluation Methodology

The subject structure was evaluated using information gathered from site observations, available historic construction documents, and interviews with District staff. This information was then utilized to perform a structural evaluation as outlined in the American Society of Civil Engineer's "Seismic Evaluation and Retrofit of Existing Buildings – ASCE 41-17" (ASCE 41-17). ASCE 41-17 is referenced as the standard for seismic evaluations of existing buildings by the International Existing Building Code (IEBC) which is referenced by the Oregon Structural Specialty Code (OSSC). Further, ASCE 41-17 is the evaluation tool required by the Seismic Rehabilitation Grant Program for grant applications.

ASCE 41-17 provides several levels of evaluation (Tiers 1-3) depending on the level of evaluation and/or retrofit being performed. The Tier 1 evaluation is a quick checklist selected based on the type of construction and the performance objective of the building and is the baseline tool for preliminary seismic evaluations. In the case of this evaluation, a Tier 1 was performed to identify the likely structural deficiencies requiring retrofit to meet the performance objective stated below.

The OSSC classifies buildings into risk categories based on the type of building and occupancy type. The building's risk category informs the required performance objective post retrofit. Risk categories I and II cover low risk structures. Risk category III includes school buildings that are not required to be used as emergency shelters and are relatively low occupancy. Risk category IV includes emergency service buildings and school buildings that are required to be designed as emergency shelters (high occupancy spaces). Figure 2, below, identifies the performance objective for each risk category.

The primary objective of the adjusting performance objectives relative to risk category is to ensure that the subject building is capable of performing in the necessary manner following a seismic event. In the case of a risk category III building, the intention is to ensure that the building is adequately stable following an earthquake to provide egress for occupants out of the building. Prior to reoccupation, the building would need evaluated and significant structural damage preventing reoccupation may be present. For risk category IV structures, the intent is that the building can be inspected then immediately reoccupied following a seismic event to function in its intended role as an emergency service building or as a high occupancy space capable of acting as an emergency structure.

In accordance with the table below, these section(s) D, E, and H of this building are categorized as a risk category IV structure(s) and were evaluated to meet the Life Safety structural performance and Hazards Reduced nonstructural performance level for BSE-2E loading and the Immediate Occupancy structural performance and Position Retention nonstructural performance level for BSE-1E loading.

Table 2-2. Scope of Assessment Required for Tier 1 and Tier 2 with the Basic Performance Objective for Existing **Buildings (BPOE)**

	Tier 1 and 2 ^a									
Risk Category	BSE-1E	BSE-2E								
I and II	Not evaluated	Collapse Prevention Structural Performance								
	Life Safety Nonstructural Performance (3-C)	Hazards Reduced Nonstructural Performance ^b (5-D)								
III	Not evaluated	Limited Safety Structural Performance ^c								
	Position Retention Nonstructural Performance (2-B)	Hazards Reduced Nonstructural Performance ^b (4-D)								
IV	Immediate Occupancy Structural Performance	Life Safety Structural Performance ^d								
	Position Retention Nonstructural Performance (1-B)	Hazards Reduced Nonstructural Performance ^b (3-D)								

^a For Tier 1 and 2 assessments of Risk Categories I-III, Structural Performance for the BSE-1E is not explicitly

evaluated. ^b Compliance with ASCE 7 provisions for new construction is

^b Compliance with ASCE 7 provisions for new construction is deemed to comply.
 ^c For Risk Category III, the Tier 1 screening checklists shall be based on the Collapse Prevention Performance Level (S-5), except that checklist statements using the Quick Check procedures of Section 4.4.3 shall be based on *M_s* factors taken as the average of the values for Life Safety and Collapse Prevention.
 ^d For Risk Category IV, the Tier 1 screening checklists shall be based on the Collapse Prevention Performance Level (S-5), except that checklist statements using the Quick Check procedures of Section 4.4.3 shall be based on *M_s* factors for Life Safety.

Figure 2

Building Performance Objectives

Source: Table 2-2, ASCE 41-17: American Society of Civil Engineers – Seismic Evaluation and Retrofit of Existing Buildings

5.0 Seismicity

Seismic design is based on site specific parameters that relate to the location of the building relative to faults and the soil that supports the building. The United States Geologic Survey has developed seismic design data that is utilized to perform the calculations specified in ASCE 41-17. The table below summarizes the factors appropriate for computing the seismic lateral loads for the design earthquake specified in ASCE 41-17.

SITE SPECIFIC SEISMICITY	
Soil Density	Stiff Soil
ASCE 7-16 Soil Classification	D
BSE-1E:	
S _{xs}	0.164
S _{x1}	0.047
BSE-2E:	
S _{xs}	0.417
S _{x1}	0.318
Soil Condition Amplification Factors (Fv, FA)	$F_v = 2.4 - F_a = 1.6$
ASCE 41 Site Seismicity	High

Source: SEAOC and OSHPD Seismic Design Maps, https://seismicmaps.org/

6.0 Site Specific Hazards

Site specific hazards were assessed as part of our engineering evaluation. The hazards evaluated in our analysis included liquefaction, slope failure, surface fault rupture, and tsunami potential. These potential hazards were evaluated using ASCE 41-17 guidelines, as well as information provided by the online Oregon HazVu: Statewide Geohazards Viewer, maintained by the Department of Geology and Mineral Industries (DOGAMI). Tsunami risk was evaluated using the ASCE Tsunami Hazard Tool. Results from the HazVu analysis are included in Appendix D along with a geotechnical report. Unless noted below, the hazards listed above are not present at the site.

Liquefaction

This project is located within a liquefaction hazard area as identified by the DOGAMI Oregon HazVu. To ensure that an acceptable level of due diligence was performed during the application phase of the project a geotechnical from a prior project at this site was reviewed for available information with respect to the severity. Per the geotechnical report, attached in Appendix B, liquefaction is considered a low risk for the site and no mitigation is required.

7.0 Deficiencies and Repairs

The table below summarizes both the structural and nonstructural deficiencies noted in the Tier 1 evaluation and states both the proposed retrofit methodology and the plan key note that corresponds to the scope items in the preliminary plans and the cost estimate. See Appendix B for complete Tier 1 check sheets. Drawings illustrating the proposed retrofit measures are attached in Appendix C.

Tier 1	Deficiency Statement	Ponair Statement	Plan Key
Deficiency Description	Deficiency Statement	Repair Statement	Note
LOAD PATH	The structure does not contain a complete, well-defined load path, including structural elements and connections, that serves to transfer the inertial forces associated with the mass of all elements of the building to the foundation.	Provide a complete, well-defined load path by installing new elements and connections as needed to transfer inertial forces from all elements of the building to the foundation.	\$1
ADJACENT BUILDINGS	The clear distance between the building being evaluated and any adjacent building is less than 0.5% of the height of the shorter building in low seismicity, 1.0% in moderate seismicity, and 3.0% in high seismicity.	Provide seismic isolation joint to avoid pounding of the taller structure into the lower structure. Provide all new gravity framing and lateral resisting elements as necessary to provide building separation. Provide new beam connections and ledgers that can accommodate the required differential out-of-plane movement while transferring gravity and in-plane lateral forces as needed.	S2
MEZZANINES	Interior mezzanine levels are not braced independently from the main structure or are not anchored to the seismic- force-resisting elements of the main structure.	Provide an independent bracing system or anchor the mezzanine to the seismic-force-resisting elements of the main structure.	S3
SHEAR STRESS CHECK	The shear stress in the unreinforced masonry shear walls, calculated using the Quick Check procedure of Section 4.4.3.3, is greater than 30lb/in.2 for clay units and 70lb/in.2 for concrete units.	Provide new vertical lateral resisting elements.	S4

Redmond School District

Tumalo Elementary School Seismic Evaluation

WALL	Exterior concrete or masonry walls that	Install new out-of-plane	
ANCHORAGE	are dependent on the diaphragm for lateral support are not anchored for out-of-plane forces at each diaphragm level with steel anchors, reinforcing dowels, or straps that are developed	anchorage.	
	into the diaphragm. Connections do not have strength to resist the connection force calculated in the Quick Check procedure of Section 4.4.3.7.		S5
WOOD LEDGERS	The connection between the wall panels and the diaphragm induces cross-grain bending or tension in the wood ledgers.	Install new out-of-plane anchorage.	S6
TRANSFER TO SHEAR WALLS	Diaphragms are not connected for transfer of seismic forces to the shear walls, or the connections are not able to develop the shear strength of the walls or diaphragms.	Install new hardware for transfer of seismic forces from diaphragm to shear walls.	S7
PLAN IRREGULARITIES	There is not tensile capacity to develop the strength of the diaphragm at reentrant corners or other locations of plan irregularities.	Provide new drag elements.	S8
STRAIGHT SHEATHING	Not all straight-sheathed diaphragms have aspect ratios less than 1-to-1 in the direction being considered.	Install new plywood diaphragm sheathing.	S9
SPANS	Not all wood diaphragms with spans greater than 12 ft consist of wood structural panels or diagonal sheathing.	Install new plywood diaphragm sheathing.	S10
DIAGONALLY SHEATHED AND UNBLOCKED DIAPHRAGMS	Not all diagonally sheathed or unblocked wood structural panel diaphragms have horizontal spans less than 30 ft and aspect ratios less than or	Install new blocked plywood diaphragm.	
STIFFNESS OF WALL ANCHORS	equal to 3-to-1. Anchors of concrete or masonry walls to wood structural elements are not installed taut or are not stiff enough to limit the relative movement between the wall and the diaphragm to no greater than 1/8 in. (3 mm) before engagement of the anchors.	Install new out-of-plane anchorage.	S11 S12
BEAM, GIRDER, AND TRUSS SUPPORTS	Beams, girders, and trusses supported by unreinforced masonry walls or pilasters do not have independent secondary columns for support of vertical loads.	Install new secondary support for vertical load carrying framing elements.	\$12 \$13
FLEXIBLE COUPLINGS	Fire suppression piping does not have flexible couplings in accordance with NFPA-13.	Install flexible couplings for fire suppression piping in accordance with NFPA-13.	N1

Tumalo Elementary School Seismic Evaluation

HAZARDOUS MATERIAL	Piping or ductwork conveying hazardous materials is not braced or	Brace piping or ductwork conveying hazardous materials.	
DISTRIBUTION	otherwise protected from damage that would allow hazardous material release.		N2
SHUTOFF VALVES	Piping containing hazardous material, including natural gas, does not have shut off valves or other devices to limit spills or leaks.	Install shut off valves for piping containing hazardous material, including natural gas.	N3
FLEXIBLE COUPLINGS	Hazardous material ductwork and piping, including natural gas piping, do not have flexible couplings.	Install flexible couplings for ductwork and piping containing hazardous material, including natural gas piping.	N4
PIPING OR DUCTS CROSSING SEISMIC	Piping or ductwork carrying hazardous material that either crosses seismic joints or isolation planes or is connected to independent structures does not have couplings or other details to accommodate the relative seismic displacements.	Install seismic joint couplings for piping or ductwork carrying hazardous material.	
UNREINFORCED MASONRY	Unreinforced masonry or hollow-clay tile partitions are not braced at a spacing of at most 10 ft in Low or Moderate Seismicity, or at most 6 ft in High Seismicity.	Brace unreinforced masonry or hollow-clay tile partitions.	N5 N6
LENS COVERS	Lens covers on light fixtures are not attached with safety devices.	Install safety devices for light fixture lens covers.	N7
INDUSTRIAL STORAGE RACKS	Industrial storage racks or pallet racks more than 12 ft high do not meet the requirements of ANSI/RMI MH 16.1 as	Provide bracing and anchorage of storage racks.	
TALL NARROW CONTENTS	modified by ASCE 7, Chapter 15. Contents more than 6 ft high with a height-to-depth or height-to-width ratio greater than 3-to-1 are not anchored to the structure or to each other.	Anchor contents to the structure.	N8
FALL-PRONE CONTENTS	Equipment, stored items, or other contents weighing more than 20lb whose center of mass is more than 4 ft above the adjacent floor level are not braced or otherwise restrained.	Brace equipment to structure.	N9 N10
FALL-PRONE EQUIPMENT	Equipment weighing more than 20 lb whose center of mass is more than 4 ft above the adjacent floor level, and which is not in-line equipment, is not braced.	Brace and anchor equipment weighing more than 20 lb, whose center of mass is more than 4 ft above the adjacent floor level.	N11
IN-LINE EQUIPMENT	Equipment installed in line with a duct or piping system, with an operating weight more than 75 lb, is not supported or laterally braced	Independently support and laterally brace equipment with an operating weight more than 75 lb installed in line with a duct or piping system.	N12

Tumalo Elementary School Seismic Evaluation

	independent of the duct or piping system.		
FLEXIBLE COUPLINGS	Fluid and gas piping does not have flexible couplings.	Install flexible couplings for fluid and gas piping.	N13
FLUID AND GAS PIPING	Fluid and gas piping is not anchored or braced to the structure to limit spills or leaks.	Anchor and brace fluid and gas piping to the structure.	N14
PIPING CROSSING SEISMIC JOINTS	Piping that crosses seismic joints or isolation planes or is connected to independent structures does not have couplings or other details to accommodate the relative seismic displacements.	Install couplings for piping that crosses seismic joints or isolation planes or is connected to independent structures.	N15

In addition to the structural and nonstructural deficiencies noted above, the gravity load resisting system was reviewed to identify obvious insufficient gravity components. Insufficient gravity elements can cause failure during seismic events. These gravity deficiencies are based on visual observations of the existing structural elements. No formal structural analysis was performed during this evaluation of the gravity resisting element.

Existing glue laminated arches built prior to 1970 were under designed based on inadequate material stress information available at the time. This results in the arch's inability to support code prescribed gravity loading. The arches will be retrofit and strengthened to support code required gravity loading. This is deficiency/repair/plan note S14.

Based upon ZCS's previous experience and discussions with site personnel the buildings contain hazardous materials. These materials will need to be dealt with on a case-by-case basis as they are encountered during the project.

8.0 Preliminary Construction Cost Estimate

The attached engineer's opinion of probable cost has been developed by ZCS. ZCS has a successful record of completing seismic rehabilitation projects within the State of Oregon. The prices provided in the attached cost estimate have been developed using the extensive list of past projects as a baseline for this project. These prices are based on Oregon BOLI wage rates. The cost estimate is broken down into multiple line items associated with each major task (general conditions, foundation, structural steel, MEP, etc) associated with the rehabilitation. Additional line items are included for design associated permit costs, and owner construction management. A complete breakdown of the cost estimate can be found in Appendix E.

Special Notes

• This building is an unreinforced masonry structure. Accordingly, it is acknowledged that a Tier 3 evaluation is required prior to the retrofit design. The consultant costs for the Tier 3 evaluation have been included in the cost estimate as a separate line item.

DIREC	CT COST
Construction	\$1,839,700
Engineering	\$289,400
Construction Management	\$ 61,000
Relocation	\$26,500
Construction Contingency	\$265,275
TOTALS AN	D SUMMARY
Total Cost Estimate	\$2,481,875
Match Funds	\$0
Total Amount Requested from SRGP	\$2,481,875
Total Area	16,000
Cost/Square Foot	\$159.09 / SF

9.0 Conclusion and Certification Statement

The findings described in this report have been limited to the lateral force-resisting structural system and general assessment of the gravity force-resisting elements. Based on our visual observations, we find the structure to be in relatively good condition and generally safe for occupancy. No significant damage to the existing structural system was discovered.

Given the current condition of the structure, the current code section on existing buildings does not mandate that upgrades are required unless the building is scheduled for repairs, alterations, additions, or change in occupancy. To clarify, upgrades outlined in this report are strictly at the discretion of the District.

Please contact our office if you would like to discuss our findings. Please review the attached schematic drawings that can be used to refine a scope and budget.

Certification Statement

ZCS Engineering & Architecture's professional staff has reviewed the subject building and the deficiencies noted in the Tier 1 evaluation, developed seismic retrofit solutions to rectify the deficiencies, and developed the engineering cost estimate. The project cost estimate was developed by ZCS based on unit costs from our extensive list of past seismic retrofit projects as a baseline. We certify to the best of our knowledge, based on known and readily identifiable existing conditions, that all the seismic deficiencies present in the building are included in the retrofit scope of work and that all the retrofit's scope of work elements are included in the cost estimate.

Matthew R. Smith, PE, SE

Redmond School District Tumalo Elementary School Seismic Evaluation

February 2022 Project No: P-2706-21

Appendix A: Figures





Figure 1: NORTH ENTRANCE



Figure 2: NORTH ELEVATION



Figure 3: BUILDING 'D' NORTH ELEVATION



Figure 4: CAFETERIA NORTH ELEVATION





Figure 5: INTERIOR HALLWAY

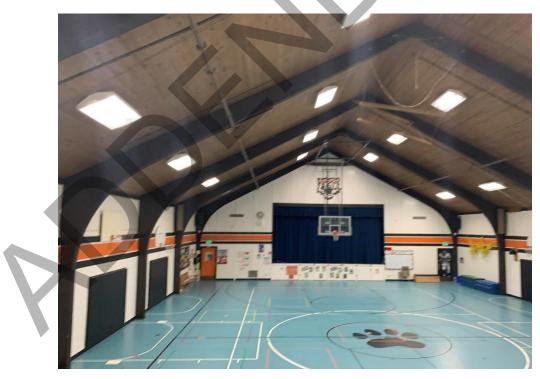


Figure 6: GYMNASIUM INTERIOR

Redmond School District Tumalo Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix B: Tier 1 Check Sheets

ASCE 41-17 Tier 1 Checklists

FIRM:	
PROJECT NAME:	
SEISMICITY LEVEL:	
PROJECT NUMBER:	
COMPLETED BY:	
DATE COMPLETED:	
REVIEWED BY:	
REVIEW DATE:	

Project Name
Project Number

17.1.210 Basic Configuration Checklist

Table 17-3. Immediate Occupancy Basic Configuration Checklist

					Tier 2	Commentary
Status	5			Evaluation Statement	Reference	Reference Comments
Very L	.ow Seis	micity				
Buildi	ng Syste	m—Gene	eral			
С	NC	N/A	U	LOAD PATH: The structure	5.4.1.1	A.2.1.1
				contains a complete, well-defined		
				load path, including structural		
				elements and connections, that		
				serves to transfer the inertial forces		
				associated with the mass of all		
				elements of the building to the		
				foundation.		
С	NC	N/A	U	ADJACENT BUILDINGS: The clear	5.4.1.2	A.2.1.2
				distance between the building		
				being evaluated and any adjacent		
				building is greater than 0.5% of		
				the height of the shorter building		
				in low seismicity, 1.0% in moderate		
				seismicity, and 3.0% in high		
				seismicity.		
С	NC	N/A	U	MEZZANINES: Interior mezzanine	5.4.1.3	A.2.1.3
				levels are braced independently		
				from the main structure or are		
				anchored to the seismic-force-		
				resisting elements of the main		
				structure.		
Buildiı	ng Syste	m—Buila	ling Co	nfiguration		
С	NC	N/A	U	WEAK STORY: The sum of the shear	5.4.2.1	A.2.2.2
				strengths of the seismic-force-		
				resisting system in any story in		
				each direction is not less than 80%		
				of the strength in the adjacent		
			,	story above.		
С	NC	N/A	U	SOFT STORY: The stiffness of the	5.4.2.2	A.2.2.3
				seismic-force-resisting system in		
				any story is not less than 70% of		
				the seismic-force-resisting system		
				stiffness in an adjacent story above		
				or less than 80% of the average		
				seismic-force-resisting system		
				stiffness of the three stories above.		
С	NC	N/A	U	VERTICAL IRREGULARITIES: All	5.4.2.3	A.2.2.4
				vertical elements in the seismic-		
				force-resisting system are		
				continuous to the foundation.		

						Project Name Project Numb	
						riojectivanic	
					<u> </u>	4.2.2.5	
C	NC	N/A	U	GEOMETRY: There are no changes in the net horizontal dimension of	5.4.2.4	A.2.2.5	
				the seismic-force-resisting system			
				of more than 30% in a story			
				relative to adjacent stories,			
				excluding one-story penthouses and mezzanines.			
	NC	N/A	U	MASS: There is no change in	5.4.2.5	A.2.2.6	
			-	effective mass of more than 50%			
				from one story to the next. Light			
				roofs, penthouses, and			
				mezzanines need not be considered.			
С	NC	N/A	U	TORSION: The estimated distance	5.4.2.6	A.2.2.7	
				between the story center of mass			
				and the story center of rigidity is			
				less than 20% of the building			
				width in either plan dimension.			*
					Tier 2	Commentary	
Status	6			Evaluation Statement	Tier 2 Reference	Commentary Reference	Comments
		y (Comp	lete the	Evaluation Statement e Following Items in Addition to the	Reference	Reference	
Low S	eismicit	:y (Comp Hazards	lete the		Reference	Reference	
Low S	eismicit		lete the	e Following Items in Addition to the LIQUEFACTION: Liquefaction-	Reference	Reference	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the	Reference Items for Very	Reference Low Seismicity)	
Low S Geolo	eismicit gic Site I	Hazards		E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site	Reference Items for Very	Reference Low Seismicity)	
Low S	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	N/A	U 	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	N/A	U 	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	N/A	U 	e Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	Hazards N/A	U 	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure.	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	N/A	U 	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface	Reference Hers for Very	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface fault rupture and surface	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	
Low S	eismicit gic Site I NC	Hazards N/A	U	E Following Items in Addition to the LIQUEFACTION: Liquefaction- susceptible, saturated, loose granular soils that could jeopardize the building's seismic performance do not exist in the foundation soils at depths within 50 ft (15.2 m) under the building. SLOPE FAILURE: The building site is located away from potential earthquake-induced slope failures or rockfalls so that it is unaffected by such failures or is capable of accommodating any predicted movements without failure. SURFACE FAULT RUPTURE: Surface	Reference Items for Very 5.4.3.1	Reference (Low Seismicity) A.6.1.1	

Project Name	
Project Number	

Status	;			Evaluation Statement	Tier 2 Reference	Commentary Reference	Comments
Moder	rate and	High Sei	smicity	y (Complete the Following Items in)	Addition to th		Seismicity)
Found	ation Co	nfigurati	on				
c		N/A	U	OVERTURNING: The ratio of the least horizontal dimension of the seismic-force-resisting system at the foundation level to the building height (base/height) is greater than 0.6 <i>S</i> _a .	5.4.3.3	A.6.2.1	N
c		N/A		TIES BETWEEN FOUNDATION ELEMENTS: The foundation has ties adequate to resist seismic forces where footings, piles, and piers are not restrained by beams, slabs, or soils classified as Site Class A, B, or C.	5.4.3.4	A.6.2.2	

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Project Name Areas 'D', 'E', 'H' Project Number

17.18IO Structural Checklist for Building Types URM: Unreinforced Masonry Bearing Walls with Flexible Diaphragms and URMa: Unreinforced Masonry Bearing Walls with Stiff Diaphragms

				Tier 2	Commentary	
s			Evaluation Statement	Reference	Reference	Comments
Low Se	eismici	ty				
ic-For	ce-Resi	sting S	System			
NC	N/A	U	REDUNDANCY: The number of lines of	5.5.1.1	A.3.2.1.1	
			shear walls in each principal direction		1	
			is greater than or equal to 2.			
NC	N/A	U	SHEAR STRESS CHECK: The shear	5.5.3.1.1	A.3.2.5.1	
			stress in the unreinforced masonry			
			shear walls, calculated using the Quick			
			•			
			less than 30 lb/in. ² (0.21 MPa) for clay			
			units and 70 lb/in. ² (0.48 MPa) for			
			concrete units.			
NC	N/A	U		5.7.1.1	A.5.1.1	
\square		\square				
			_			
NC	N/A	U		5.7.1.3	A.5.1.2	
NC	NI/A		-	570	A E O 1	
NC	IN/A	0		5.7.2	A.J.Z.I	
NC	N/A	U	GIRDER-COLUMN CONNECTION:	5.7.4.1	A.5.4.1	
inc.	IN/A	0	There is a positive connection using	J./.T.I	1	
			plates, connection hardware, or straps between the girder and the column			
	ow Se ic-Ford NC NC	ow Seismicit ic-Force-Resi NC N/A NC N/A Constructions NC N/A NC N/A NC N/A NC N/A NC N/A Image: NC N/A Image: NC N/A Image: NC N/A Image: NC N/A	.ow Seismicity ic-Force-Resisting S NC N/A U U NC N/A NC N/A U U Sections NC N/A NC N/A U U NC N/A U U U U	Jow Seismicity ic-Force-Resisting System NC N/A U REDUNDANCY: The number of lines of shear walls in each principal direction is greater than or equal to 2. NC N/A U SHEAR STRESS CHECK: The shear Image: Stress in the unreinforced masonry shear walls, calculated using the Quick Check procedure of Section 4.4.3.3, is less than 30 lb/in. ² (0.21 MPa) for clay units and 70 lb/in. ² (0.48 MPa) for concrete units. PC N/A U WALL ANCHORAGE: Exterior concrete or masonry walls that are dependent on the diaphragm for lateral support are anchored for out-of-plane forces at each diaphragm level with steel anchors, reinforcing dowels, or straps that are developed into the diaphragm. Connections have strength to resist the connection force calculated in the Quick Check procedure of Section 4.4.3.7. NC N/A U WOOD LEDGERS: The connection force calculated in the Quick Check procedure of Section 4.4.3.7.	s Evaluation Statement Reference c.ow Seismicity REDUNDANCY: The number of lines of s.5.1.1 5.5.1.1 Image: Set of the second set of the secon	s Evaluation Statement Reference Reference c.ow Seismicity

Table 17-37. Immediate Occupancy Structural Checklist for Building Types URM and URMa

Project Name Areas 'D', 'E', 'F'
Project Number

Foundation System **DEEP FOUNDATIONS: Piles and piers** A.6.2.3 С NC N/A U are capable of transferring the lateral forces between the structure and the soil. N/A SLOPING SITES: The difference in С NC U A.6.2.4 foundation embedment depth from one side of the building to another does not exceed one story high. Commentary Tier 2 Status **Evaluation Statement** Reference Reference Comments Low, Moderate, and High Seismicity (Complete the Following Items in Addition to the Items for Very Low Seismicity) Seismic-Force-Resisting System NC PROPORTIONS: The height-to-5.5.3.1.2 A.3.2.5.2 С N/A υ thickness ratio of the shear walls at \square each story is less than the following: Top story of multi-story building 9 First story of multi-story building 15 All other conditions 13 NC N/A 5.5.3.4.1 A.3.2.5.3 С MASONRY LAYUP: Filled collar joints of U multi-wythe masonry walls have negligible voids. Diaphragms (Stiff or Flexible) NC N/A **OPENINGS AT SHEAR WALLS:** 5.6.1.3 A.4.1.4 С υ Diaphragm openings immediately adjacent to the shear walls are less than 15% of the wall length. NC N/A υ **OPENINGS AT EXTERIOR MASONRY** 5.6.1.3 A.4.1.6 С SHEAR WALLS: Diaphragm openings immediately adjacent to exterior masonry shear walls are not greater than 4 ft (1.2 m) long. NC N/A PLAN IRREGULARITIES: There is tensile С U 5.6.1.4 A.4.1.7 capacity to develop the strength of the diaphragm at reentrant corners or other locations of plan irregularities. С NC N/A U DIAPHRAGM REINFORCEMENT AT 5.6.1.5 A.4.1.8 **OPENINGS:** There is reinforcing around ٦ all diaphragm openings larger than 50% of the building width in either major plan dimension. Flexible Diaphragms С NC N/A U **CROSS TIES: There are continuous** 5.6.1.2 A.4.1.2 cross ties between diaphragm chords.

						Project Name	Areas 'D', 'E', 'H'
						Project Number	
с	NC	N/A	U	STRAIGHT SHEATHING: All straight-	5.6.2	A.4.2.1	
				sheathed diaphragms have aspect			
				ratios less than 1-to-1 in the direction			
с	NC	N/A	U	being considered. SPANS: All wood diaphragms with	5.6.2	A.4.2.2	
C		IN/A	Ū	spans greater than 12 ft (3.6 m) consist	5.0.2	A.4.2.2	
				of wood structural panels or diagonal			
				sheathing.			
С	NC	N/A	U	DIAGONALLY SHEATHED AND	5.6.2	A.4.2.3	
				UNBLOCKED DIAPHRAGMS: All			
				diagonally sheathed or unblocked wood structural panel diaphragms			
				have horizontal spans less than 30 ft			
				(9.2 m) and aspect ratios less than or			
				equal to 3-to-1.			
С	NC	N/A	U	NONCONCRETE FILLED DIAPHRAGMS:	5.6.3	A.4.3.1	
				Untopped metal deck diaphragms or			
				metal deck diaphragms with fill other			
				than concrete consist of horizontal spans of less than 40 ft (12.2 m) and			
				have aspect ratios less than 4-to-1.			
с	NC	N/A	U	OTHER DIAPHRAGMS: Diaphragms do	5.6.5	A.4.7.1	
				not consist of a system other than			
				wood, metal deck, concrete, or			
				horizontal bracing.			
Conn	ection			STIFFNESS OF WALL ANCHORS:	5.7.1.2	A.5.1.4	
ر 	NC	N/A	U	Anchors of concrete or masonry walls	5.7.1.2	A.J.1.4	
				to wood structural elements are			
				installed taut and are stiff enough to			
				limit the relative movement between			
				the wall and the diaphragm to no			
				greater than 1/8 in. (3 mm) before			
~	NC	N/A	U	engagement of the anchors. BEAM, GIRDER, AND TRUSS SUPPORTS:	5711	A.5.4.5	
c	NC	IN/A	0	Beams, girders, and trusses supported	5.7.4.4	A.J.4.J	
				by unreinforced masonry walls or			
				pilasters have independent secondary			
				columns for support of vertical loads.			
				· · · · · · · · · · · · · · · · · · ·			
			Ū				

Areas 'D', 'E', 'H' Project Name

Project Number

17.19 Nonstructural Checklist

Table 17-38. Nonstructural Checklist

Status			Evaluation Statement ^{a,b}	Tier 2 Reference	Commentary Reference	Comments
Life Safety S	ystem	5				
	N/A	, U	HR—not required; LS—LMH; PR—LMH. FIRE	13.7.4	A.7.13.1	
			SUPPRESSION PIPING: Fire suppression piping is			
			anchored and braced in accordance with NFPA-13.			
C NC	N/A	U	HR—not required; LS—LMH; PR—LMH. FLEXIBLE	13.7.4	A.7.13.2	
		Ū	COUPLINGS: Fire suppression piping has flexible	13.7.1	1.07.13.2	
			couplings in accordance with NFPA-13.		*	
C NC	N/A	U	HR—not required; LS—LMH; PR—LMH.	13.7.7	A.7.12.1	
		Č	EMERGENCY POWER: Equipment used to power or	13.7.7	,	
			control Life Safety systems is anchored or braced.			
C NC	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR AND	13.7.6	A.7.14.1	
		-	SMOKE DUCTS: Stair pressurization and smoke			
			control ducts are braced and have flexible			
			connections at seismic joints.			
C NC	N/A	U	HR—not required; LS—MH; PR—MH. SPRINKLER	13.7.4	A.7.13.3	
		-	CEILING CLEARANCE: Penetrations through panelized			
			ceilings for fire suppression devices provide			
			clearances in accordance with NFPA-13.			
C NC	N/A	U	HR—not required; LS—not required; PR—LMH.	13.7.9	A.7.3.1	
			EMERGENCY LIGHTING: Emergency and egress			
			lighting equipment is anchored or braced.			
Hazardous	Materio	als				
C NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. HAZARDOUS	13.7.1	A.7.12.2	
			MATERIAL EQUIPMENT: Equipment mounted on			
			vibration isolators and containing hazardous material			
			is equipped with restraints or snubbers.			
C NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. HAZARDOUS	13.8.3	A.7.15.1	
			MATERIAL STORAGE: Breakable containers that hold			
			hazardous material, including gas cylinders, are			
			restrained by latched doors, shelf lips, wires, or other			
			methods.			
C NC	N/A	U	HR—MH; LS—MH; PR—MH. HAZARDOUS MATERIAL	13.7.3	A.7.13.4	
			DISTRIBUTION: Piping or ductwork conveying	13.7.5		
			hazardous materials is braced or otherwise protected			
			from damage that would allow hazardous material			
			release.			
C NC	N/A	U	HR—MH; LS—MH; PR—MH. SHUTOFF VALVES:	13.7.3	A.7.13.3	
			Piping containing hazardous material, including	13.7.5		
			natural gas, has shutoff valves or other devices to			
	W		limit spills or leaks.			
C NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. FLEXIBLE	13.7.3	A.7.15.4	
			COUPLINGS: Hazardous material ductwork and	13.7.5		
			piping, including natural gas piping, have flexible			

					Project Name	Areas 'D', 'E', 'H'
					Project Numbe	
	NC	N/A	U	HR—MH; LS—MH; PR—MH. PIPING OR DUCTS	13.7.3 A.7	.13.6
ر 		N/A	0	CROSSING SEISMIC JOINTS: Piping or ductwork	13.7.5 A.7	.15.0
				carrying hazardous material that either crosses	13.7.6	
				seismic joints or isolation planes or is connected to	13.7.0	
				independent structures has couplings or other details		
				to accommodate the relative seismic displacements.		
Parti	tions					
C	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. UNREINFORCED	13.6.2 A.7	.1.1
				MASONRY: Unreinforced masonry or hollow-clay tile		
				partitions are braced at a spacing of at most 10 ft (3.0		
				m) in Low or Moderate Seismicity, or at most 6 ft (1.8		
				m) in High Seismicity.		
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. HEAVY PARTITIONS	13.6.2 A.7	.2.1
				SUPPORTED BY CEILINGS: The tops of masonry or		
				hollow-clay tile partitions are not laterally supported		
				by an integrated ceiling system.		
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. DRIFT: Rigid	13.6.2 A.7	.1.2
				cementitious partitions are detailed to accommodate		
				the following drift ratios: in steel moment frame,		
				concrete moment frame, and wood frame buildings,		
				0.02; in other buildings, 0.005.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2 A.7	.2.1
				LIGHT PARTITIONS SUPPORTED BY CEILINGS: The tops		
				of gypsum board partitions are not laterally		
				supported by an integrated ceiling system.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2 A.7	.1.3
	\square			STRUCTURAL SEPARATIONS: Partitions that cross		
				structural separations have seismic or control joints.		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.2 A.7	.1.4
				TOPS: The tops of ceiling-high framed or panelized		
				partitions have lateral bracing to the structure at a		
				spacing equal to or less than 6 ft (1.8 m).		
Ceilir	-					
С	NC	N/A	U	HR—H; LS—MH; PR—LMH. SUSPENDED LATH AND	13.6.4 A.7	.2.3
				PLASTER: Suspended lath and plaster ceilings have		
			_	attachments that resist seismic forces for every 12 ft ²		
	~			(1.1 m ²) of area.		
С	NC	N/A	U	HR—not required; LS—MH; PR—LMH. SUSPENDED	13.6.4 A.7	.2.3
				GYPSUM BOARD: Suspended gypsum board ceilings		
				have attachments that resist seismic forces for every		
				12 ft ² (1.1 m ²) of area.		

Project Name Areas 'D', 'E', 'H'
Project Number

C	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.2
				INTEGRATED CEILINGS: Integrated suspended ceilings		
				with continuous areas greater than 144 ft ² (13.4 m ²)		
				and ceilings of smaller areas that are not surrounded		
				by restraining partitions are laterally restrained at a		
				spacing no greater than 12 ft (3.6 m) with members		
				attached to the structure above. Each restraint		
				location has a minimum of four diagonal wires and		
				compression struts, or diagonal members capable of		
~	NC	NI / A		resisting compression.	1264	
C	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.4
				EDGE CLEARANCE: The free edges of integrated suspended ceilings with continuous areas greater		
				than 144 ft ² (13.4 m ²) have clearances from the		
				enclosing wall or partition of at least the following: in		
				Moderate Seismicity, 1/2 in. (13 mm); in High		
				Seismicity, 3/4 in. (19 mm).		
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.4	A.7.2.5
				CONTINUITY ACROSS STRUCTURE JOINTS: The ceiling		
				system does not cross any seismic joint and is not		
				attached to multiple independent structures.		
С	NC	N/A	U	HR—not required; LS—not required; PR—H. EDGE	13.6.4	A.7.2.6
				SUPPORT: The free edges of integrated suspended		
				ceilings with continuous areas greater than 144 ft ²	/	
				(13.4 m ²) are supported by closure angles or channels		
				not less than 2 in. (51 mm) wide.		
C	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.6.4	A.7.2.7
				SEISMIC JOINTS: Acoustical tile or lay-in panel ceilings		
				have seismic separation joints such that each		
				continuous portion of the ceiling is no more than		
				2,500 ft ² (232.3 m ²) and has a ratio of long-to-short		
licht	Fixtur			dimension no more than 4-to-1.		
C	NC	N/A	U	HR—not required; LS—MH; PR—MH.	13.6.4	A.7.3.2
с —		N/A	0	INDEPENDENT SUPPORT: Light fixtures that weigh	13.0.4	A.7.3.2
				more per square foot than the ceiling they penetrate	13.7.2	
				are supported independent of the grid ceiling		
				suspension system by a minimum of two wires at		
				diagonally opposite corners of each fixture.		
	1					

				Project Na Project Nu		Areas 'D', 'E', 'H'
C NC	N/A	U	HR—not required; LS—not required; PR—H. PENDANT SUPPORTS: Light fixtures on pendant supports are attached at a spacing equal to or less than 6 ft. Unbraced suspended fixtures are free to allow a 360-degree range of motion at an angle not less than 45 degrees from horizontal without contacting adjacent components. Alternatively, if rigidly supported and/or braced, they are free to move with the structure to which they are attached without damaging adjoining components. Additionally, the connection to the structure is capable of accommodating the movement without failure.	13.7.9	A.7.3.3	
C NC	N/A	U	HR—not required; LS—not required; PR—H. LENS	13.7.9	A.7.3.4	-
			COVERS: Lens covers on light fixtures are attached with safety devices.			
Cladding a	nd Glaz	ing				
C NC	N/A	U	HR—MH; LS—MH; PR—MH. CLADDING ANCHORS:	13.6.1	A.7.4.1	
			Cladding components weighing more than 10 lb/ft ² (0.48 kN/m ²) are mechanically anchored to the structure at a spacing equal to or less than the following: for Life Safety in Moderate Seismicity, 6 ft (1.8 m); for Life Safety in High Seismicity and for Position Retention in any seismicity, 4 ft (1.2 m)			
с NС	N/A	U	HR—not required; LS—MH; PR—MH. CLADDING ISOLATION: For steel or concrete moment-frame buildings, panel connections are detailed to accommodate a story drift ratio by the use of rods attached to framing with oversize holes or slotted holes of at least the following: for Life Safety in Moderate Seismicity, 0.01; for Life Safety in High Seismicity and for Position Retention in any seismicity, 0.02, and the rods have a length-to- diameter ratio of 4.0 or less.	13.6.1	A.7.4.3	3
C NC	N/A		HR—MH; LS—MH; PR—MH. MULTI-STORY PANELS: For multi-story panels attached at more than one floor level, panel connections are detailed to accommodate a story drift ratio by the use of rods attached to framing with oversize holes or slotted holes of at least the following: for Life Safety in Moderate Seismicity, 0.01; for Life Safety in High Seismicity and for Position Retention in any seismicity, 0.02, and the rods have a length-to- diameter ratio of 4.0 or less.	13.6.1	A.7.4.4	

					Project Nar		Areas 'D', 'E', 'H'
					Project Nur	nber	
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. THREADED	13.6.1	A.7.4.9	1
				RODS: Threaded rods for panel connections detailed			
				to accommodate drift by bending of the rod have a			
				length-to-diameter ratio greater than 0.06 times the			
				story height in inches for Life Safety in Moderate			
				Seismicity and 0.12 times the story height in inches			
				for Life Safety in High Seismicity and Position			
				Retention in any seismicity.			
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. PANEL CONNECTIONS:	13.6.1.4	A.7.4.5	
				Cladding panels are anchored out of plane with a			
				minimum number of connections for each wall panel,			
				as follows: for Life Safety in Moderate Seismicity, 2			
				connections; for Life Safety in High Seismicity and for			
				Position Retention in any seismicity, 4 connections.			
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. BEARING	13.6.1.4	A.7.4.6	
				CONNECTIONS: Where bearing connections are used,			
				there is a minimum of two bearing connections for			
				each cladding panel.			
С	NC	N/A	U	HR—MH; LS—MH; PR—MH. INSERTS: Where	13.6.1.4	A.7.4.7	,
	\square			concrete cladding components use inserts, the inserts			
				have positive anchorage or are anchored to			
				reinforcing steel.			
С	NC	N/A	U	HR—not required; LS—MH; PR—MH. OVERHEAD	13.6.1.5	A.7.4.8	
				GLAZING: Glazing panes of any size in curtain walls			
				and individual interior or exterior panes more than 16 f_{12}^{2} (1.5 m ²) in area are large instant of an analysis			
				ft ² (1.5 m ²) in area are laminated annealed or			
				laminated heat-strengthened glass and are detailed			
Masa	onry Ve	noor		to remain in the frame when cracked.			
C	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. TIES:	13.6.1.2	A.7.5.1	
<u> </u>			<u> </u>	Masonry veneer is connected to the backup with	15.0.1.2	R.7.J.1	
				corrosion-resistant ties. There is a minimum of one tie			
				for every $2-2/3$ ft ² (0.25 m ²), and the ties have spacing			
				no greater than the following: for Life Safety in Low or			
				Moderate Seismicity, 36 in. (914 mm); for Life Safety in			
				High Seismicity and for Position Retention in any			
				seismicity, 24 in. (610 mm).			
с	NC	N/A	υ	HR—not required; LS—LMH; PR—LMH. SHELF	13.6.1.2	A.7.5.2	
-				ANGLES: Masonry veneer is supported by shelf angles			
				or other elements at each floor above the ground			
				floor.			
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. WEAKENED	13.6.1.2	A.7.5.3	
				PLANES: Masonry veneer is anchored to the backup			
				adjacent to weakened planes, such as at the locations			
				of flashing.			

					Project Na	me	Areas 'D', 'E', 'H'
					Project Nu	ımber	
							-
C	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. UNREINFORCED	13.6.1.1	A.7.7.	2
				MASONRY BACKUP: There is no unreinforced masonry	13.6.1.2		
				backup.	12 (1 1		1
C	NC	N/A	U	HR—not required; LS—MH; PR—MH. STUD TRACKS: For veneer with cold-formed steel stud	13.6.1.1 13.6.1.2	A.7.6.	I
				backup, stud tracks are fastened to the structure at a	15.0.1.2		
				spacing equal to or less than 24 in. (610 mm) on			
				center.			
c	NC	N/A	U	HR—not required; LS—MH; PR—MH. ANCHORAGE:	13.6.1.1	A.7.7.	1
-				For veneer with concrete block or masonry backup,	13.6.1.2		
				the backup is positively anchored to the structure at a			
				horizontal spacing equal to or less than 4 ft along the			
				floors and roof.			
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.1.2	A.7.5.	5
				WEEP HOLES: In veneer anchored to stud walls, the			
				veneer has functioning weep holes and base flashing.			
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.1.1	A.7.6.	2
			\square	OPENINGS: For veneer with cold-formed-steel stud	13.6.1.2		
				backup, steel studs frame window and door			
				openings.			
				mentation, and Appendages			-
C	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. URM PARAPETS OR	13.6.5	A.7.8.	1
				CORNICES: Laterally unsupported unreinforced			
				masonry parapets or cornices have height-to-			
				thickness ratios no greater than the following: for Life Safety in Low or Moderate Seismicity, 2.5; for Life			
				Safety in High Seismicity and for Position Retention in			
				any seismicity, 1.5.			
с	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. CANOPIES:	13.6.6	A.7.8.)
-				Canopies at building exits are anchored to the			-
				structure at a spacing no greater than the following:			
				for Life Safety in Low or Moderate Seismicity, 10 ft (3.0			
				m); for Life Safety in High Seismicity and for Position			
				Retention in any seismicity, 6 ft (1.8 m).			
С	NC	N/A	U	HR—H; LS—MH; PR—LMH. CONCRETE PARAPETS:	13.6.5	A.7.8.	3
				Concrete parapets with height-to-thickness ratios			
				greater than 2.5 have vertical reinforcement.			
С	NC	N/A	U	HR—MH; LS—MH; PR—LMH. APPENDAGES:	13.6.6	A.7.8.4	4
				Cornices, parapets, signs, and other ornamentation or			
				appendages that extend above the highest point of			
				anchorage to the structure or cantilever from			
				components are reinforced and anchored to the			
				structural system at a spacing equal to or less than 6			
				ft (1.8 m). This evaluation statement item does not			
				apply to parapets or cornices covered by other			
				evaluation statements.			

Project Name Areas 'D', 'E', 'H'

Maso	onry Ch	nimneys	5			
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. URM CHIMNEYS:	13.6.7	A.7.9.1
				Unreinforced masonry chimneys extend above the		
				roof surface no more than the following: for Life		
				Safety in Low or Moderate Seismicity, 3 times the		
				least dimension of the chimney; for Life Safety in High		
				Seismicity and for Position Retention in any		
				seismicity, 2 times the least dimension of the		
				chimney.		
С	NC	N/A	U	HR—LMH; LS—LMH; PR—LMH. ANCHORAGE:	13.6.7	A.7.9.2
				Masonry chimneys are anchored at each floor level, at		
				the topmost ceiling level, and at the roof.		
Stair	5					
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR	13.6.2	A.7.10.1
				ENCLOSURES: Hollow-clay tile or unreinforced	13.6.8	
				masonry walls around stair enclosures are restrained		
				out of plane and have height-to-thickness ratios not		
				greater than the following: for Life Safety in Low or		
				Moderate Seismicity, 15-to-1; for Life Safety in High		
				Seismicity and for Position Retention in any		
				seismicity, 12-to-1.		
С	NC	N/A	U	HR—not required; LS—LMH; PR—LMH. STAIR	13.6.8	A.7.10.2
				DETAILS: The connection between the stairs and the		
				structure does not rely on post-installed anchors in		
				concrete or masonry, and the stair details are capable		
				of accommodating the drift calculated using the		
				Quick Check procedure of Section 4.4.3.1 for		
				moment-frame structures or 0.5 in. for all other		
				structures without including any lateral stiffness		
				contribution from the stairs.		
Cont	ents ar	nd Furn	ishing	s		
С	NC	N/A	U	HR—LMH; LS—MH; PR—MH. INDUSTRIAL STORAGE	13.8.1	A.7.11.1
				RACKS: Industrial storage racks or pallet racks more		
				than 12 ft high meet the requirements of ANSI/RMI		
				MH 16.1 as modified by ASCE 7, Chapter 15.		
С	NC	N/A	U	HR—not required; LS—H; PR—MH. TALL NARROW	13.8.2	A.7.11.2
				CONTENTS: Contents more than 6 ft (1.8 m) high with		
				a height-to-depth or height-to-width ratio greater		
				than 3-to-1 are anchored to the structure or to each		
				other.		
С	NC	N/A	U	HR—not required; LS—H; PR—H. FALL-PRONE	13.8.2	A.7.11.3
				CONTENTS: Equipment, stored items, or other		
				contents weighing more than 20 lb (9.1 kg) whose		
				center of mass is more than 4 ft (1.2 m) above the		
				adjacent floor level are braced or otherwise		
				restrained.		

					Project N	lame	Areas 'D', 'E', 'H'
					Project N	lumber	
	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.6.10	A.7.11	Δ
c		N/A	U	ACCESS FLOORS: Access floors more than 9 in. (229	15.0.10	A.7.11	.4
				mm) high are braced.			
С	NC	N/A	U	HR—not required; LS—not required; PR—MH.	13.7.7	A.7.11	.5
				EQUIPMENT ON ACCESS FLOORS: Equipment and	13.6.10		
				other contents supported by access floor systems are			
				anchored or braced to the structure independent of			
	NC	N/A	U	the access floor. HR—not required; LS—not required; PR—H.	13.8.2	A.7.11	6
c	NC	N/A	U	SUSPENDED CONTENTS: Items suspended without	13.8.2	A.7.11	.0
				lateral bracing are free to swing from or move with			
				the structure from which they are suspended without			
				damaging themselves or adjoining components.			
Mech	anical	and Ele	ectrica	l Equipment			
С	NC	N/A	U	HR—not required; LS—H; PR—H. FALL-PRONE	13.7.1	A.7.12	
				EQUIPMENT: Equipment weighing more than 20 lb	13.7.7		
				(9.1 kg) whose center of mass is more than 4 ft (1.2 m)			
				above the adjacent floor level, and which is not in-			
c	NC	N/A	U	line equipment, is braced. HR—not required; LS—H; PR—H. IN-LINE	13.7.1	A.7.12	E
		N/A	Ū	EQUIPMENT: Equipment installed in line with a duct	15.7.1	A.7.12	
				or piping system, with an operating weight more			
				than 75 lb (34.0 kg), is supported and laterally braced			
				independent of the duct or piping system.			
С	NC	N/A	U	HR—not required; LS—H; PR—MH. TALL NARROW	13.7.1	A.7.12	2.6
			\square	EQUIPMENT: Equipment more than 6 ft (1.8 m) high	13.7.7		
				with a height-to-depth or height-to-width ratio			
				greater than 3-to-1 is anchored to the floor slab or			
c	NC	N/A	U	adjacent structural walls. HR—not required; LS—not required; PR—MH.	13.6.9	A.7.12	7
ر 		N/A	Ū	MECHANICAL DOORS: Mechanically operated doors	15.0.9	A.7.12	/
				are detailed to operate at a story drift ratio of 0.01.			
с	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12	8
				SUSPENDED EQUIPMENT: Equipment suspended	13.7.7		
				without lateral bracing is free to swing from or move			
				with the structure from which it is suspended without			
				damaging itself or adjoining components.			
c	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12	.9
				VIBRATION ISOLATORS: Equipment mounted on			
				vibration isolators is equipped with horizontal restraints or snubbers and with vertical restraints to			
				resist overturning.			
с	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.1	A.7.12	.10
-			_	HEAVY EQUIPMENT: Floor-supported or platform-	13.7.7		
				supported equipment weighing more than 400 lb			

				Project Nar Project Nur	e	Areas 'D', 'E', 'H'
C NC	N/A	U	HR—not required; LS—not required; PR—H. ELECTRICAL EQUIPMENT: Electrical equipment is laterally braced to the structure.	13.7.7	A.7.12	.11
C NC	N/A	U	HR—not required; LS—not required; PR—H. CONDUIT COUPLINGS: Conduit greater than 2.5 in. (64 mm) trade size that is attached to panels, cabinets, or other equipment and is subject to relative seismic displacement has flexible couplings or connections.	13.7.8	A.7.12	.12
Piping						
с NC	N/A	U	HR—not required; LS—not required; PR—H. FLEXIBLE COUPLINGS: Fluid and gas piping has flexible couplings.	13.7.3 13.7.5	A.7.13	.2
C NC	N/A	U	HR—not required; LS—not required; PR—H. FLUID	13.7.3	A.7.13	.4
			AND GAS PIPING: Fluid and gas piping is anchored and braced to the structure to limit spills or leaks.	13.7.5		
C NC	N/A	U	HR—not required; LS—not required; PR—H. C-	13.7.3	A.7.13	.5
			CLAMPS: One-sided C-clamps that support piping larger than 2.5 in. (64 mm) in diameter are restrained.	13.7.5	•	
C NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.3	A.7.13	.6
			PIPING CROSSING SEISMIC JOINTS: Piping that crosses seismic joints or isolation planes or is connected to independent structures has couplings or other details to accommodate the relative seismic displacements.	13.7.5		
Ducts						
	N/A]		HR—not required; LS—not required; PR—H . DUCT BRACING: Rectangular ductwork larger than 6 ft ² (0.56 m ²) in cross-sectional area and round ducts larger than 28 in. (711 mm) in diameter are braced. The maximum spacing of transverse bracing does not exceed 30 ft (9.2 m). The maximum spacing of longitudinal bracing does not exceed 60 ft (18.3 m).	13.7.6	A.7.14	.2
C NC	N/A	U	HR—not required; LS—not required; PR—H. DUCT	13.7.6	A.7.14	.3
			SUPPORT: Ducts are not supported by piping or			
			electrical conduit.			
C NC	N/A	U	electrical conduit. HR—not required; LS—not required; PR—H.	13.7.6	A.7.14	.4
		U	electrical conduit. HR—not required; LS—not required; PR—H. DUCTS CROSSING SEISMIC JOINTS: Ducts that cross seismic joints or isolation planes or are connected to independent structures have couplings or other details to accommodate the relative seismic	13.7.6	A.7.14	.4
		U	electrical conduit. HR—not required; LS—not required; PR—H. DUCTS CROSSING SEISMIC JOINTS: Ducts that cross seismic joints or isolation planes or are connected to independent structures have couplings or other	13.7.6	A.7.14	.4
			electrical conduit. HR—not required; LS—not required; PR—H. DUCTS CROSSING SEISMIC JOINTS: Ducts that cross seismic joints or isolation planes or are connected to independent structures have couplings or other details to accommodate the relative seismic displacements. HR—not required; LS—H; PR—H. RETAINER GUARDS: Sheaves and drums have cable retainer	13.7.6	A.7.14	
Elevators	N/A		electrical conduit. HR—not required; LS—not required; PR—H. DUCTS CROSSING SEISMIC JOINTS: Ducts that cross seismic joints or isolation planes or are connected to independent structures have couplings or other details to accommodate the relative seismic displacements. HR—not required; LS—H; PR—H. RETAINER			.1

					Project Nar Project Nur		Areas 'D', 'E', 'H'
с	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.3
				ELEVATOR EQUIPMENT: Equipment, piping, and other components that are part of the elevator system are anchored.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.4
				SEISMIC SWITCH: Elevators capable of operating at speeds of 150 ft/min (0.30 m/min) or faster are			
				equipped with seismic switches that meet the			
				requirements of ASME A17.1 or have trigger levels set			
				to 20% of the acceleration of gravity at the base of			
				the structure and 50% of the acceleration of gravity in other locations.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.5
\square				SHAFT WALLS: Elevator shaft walls are anchored and			
				reinforced to prevent toppling into the shaft during			
				strong shaking.			
C	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.6
				COUNTERWEIGHT RAILS: All counterweight rails and divider beams are sized in accordance with ASME			
				A17.1.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.7
				BRACKETS: The brackets that tie the car rails and the			
				counterweight rail to the structure are sized in			
				accordance with ASME A17.1.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H.	13.7.11	A.7.16	.8
				SPREADER BRACKET: Spreader brackets are not used to resist seismic forces.			
С	NC	N/A	U	HR—not required; LS—not required; PR—H. GO-	13.7.11	A.7.16	.9
				SLOW ELEVATORS: The building has a go-slow			
				elevator system.			
Perfo	rmance	e Level:	HR = H	lazards Reduced, LS = Life Safety, and PR = Position Reter	ntion.		

^{*a*} Performance Level: HR = Hazards Reduced, LS = Life Safety, ^{*b*} Level of Seismicity: L = Low, M = Moderate, and H = High.

Redmond School District Tumalo Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix C: Schematic Seismic Retrofit Drawings

TUMALO COMMUNITY SCHOOL SEISMIC RETROFIT

PRELIMINARY DESIGN

REDMOND SCHOOL DISTRICT 19835 2ND ST. TUMALO, OREGON 97703

		TUIVIALO, C	REGON 97703	
ABBREVIATIONS		SHEET INDEX:	EXTERIOR PHOTO: NORTH ENTRANCE	EXTER
ABBREVIATIONS LP. (E) EXISTING M.C. (N) NEW M.D.F (N) NEW M.D.F (R) REMOVE M.D.G A.C. ASCHALT CONCRETE MEM A.C. ASCUSTICAL BOARD MH. A.C.F. ACOUSTICAL FOARD MH. A.C.T. ACOUSTICAL FOARD MR. A.D. AREA DRAIN M.P. AD. AREA DRAIN M.P. AGR. AGGREGATE MUL A.F. AGCESS FLOORING MTD. BD. BOARD NITS. BITUMINOUS OBS. BR/P. BACKING PLATE C.C. BD. BOTAD BOTTOMBOTTOM OF C.C.G C.G. CEM. CEMAENT O.F.D. CEM. CEMAENT O.F.D. C.G. CONTROL JOINT C.G. C.G. CONTROL JOINT O.L. C.J. CASTRON O.F.D. C.L. C	FIBERBOARD HEDIW DENSITY OVERLAY MEDIW DENSITY OVERLAY MANDUE MIROR MANDUE MANDUE MANDUE MACHINE SCREW MOUNTED MULION NOTIO SCALE OBSCURE OVERHEAD COLING DOOR OVERHEAD COLING GRILLE OVERHEAD COLING GRILLE OVERHEAD COLING SCH CONTRACTOR INSTALLED OVERTERD COLING SCH OVERHEAD COLING SCH CONTRACTOR INSTALLED OVERTERD COLING SCH OVERHEAD COLING SCH CONTRACTOR INSTALLED OVERTERNISHED WINER INSTALLED OPPOSITE HAND PLATE	G0.0 COVER SHEET A1.1 BUILDING KEY PLAN S1.1 REPAIR KEY NOTES S2.1 AREA 'D', 'E', & 'H' ROOF FRAMING PLAN		
CLO. CLOSET PLAN CLR. CLEAR PLAS. C.M. CONCRETE MASONRY UNIT P.C.P. C.O. CASED OPENING PC CONN CONRECTION PR. CORR. CORRIDOR PTN. CORT. CARPET R.C.P. CTSK. COUNTERSUNK R.D. CT. CENTER R.D. CT. CENTER R.O. DET. DRINKING FOUNTAIN RWD. DET. DETAIL R.W.L. DISP DISPENSER R.V.L. D.S. DOOR S.C. DWR. DAWER S.C. D.S. DOVNSPOUT SHR. D.S. DONNANIONT SLD. E.V. ELPANSION JOINT	PLASTER PORTLAND CEMENT PLASTER PARTITION PARTITION REFLECTED CELLING PLAN REDUCTED CELLING PLAN REDUCATE REDWOOD REDWOOD RENTLED CELLING PLAN REDWOOD REDWOO	VICINITY MAP		EXTER
G.B. GRAB BAAR	VERIFY IN FIELD	20) Bith St min Lin Constrained on the second of the secon	INTERIOR PHOTO: HALLWAY	
ROOM ROOM NAME NAME ROOM NUMBER OUSF ROOM AREA XXX DOOR NUMBER X FINISH TYPE X WALL TYPE TAG X WINDOW/GLAZING TAG	NITERIOR DRAWNS REFE SHEET REFER SHEET REFER NULDING ADD DRAWNS REF SHEET REFER SHEET REFER SHEET REFER	ENCE CCELLING HEIGHT, A.F.F. WALL SECTION ALIGN CONTINUATION CONTI	ATING	





REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND OR 97756

TUMALO COMMUNITY SCHOOL SEISMIC RETROFIT

RIOR PHOTO: NORTH ELEVATION



RIOR PHOTO: KITCHEN & CAFETERIA



RIOR PHOTO: GYMNASIUM



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524 Main Street, Suite 2, Oregon City Oregon 97045 | 503-659-2205

REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND OR 97756

TUMALO COMMUNITY SCHOOL SEISMIC RETROFIT



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PREL

STRUCTURAL REPAIRS:

- PROVIDE A COMPLETE, WELL-DEFINED LOAD PATH BY INSTALLING NEW ELEMENTS AND CONNECTIONS AS NEEDED TO TRANSFER INERTIAL FORCES FROM ALL ELEMENTS OF THE BUILDING TO THE FOUNDATION.
 - A. INSTALL DIAPHRAGM ATTACHMENTS IN-PLANE SHEAR в. PARTIAL HEIGHT MASONRY - PROVIDE SPANDREL &
- STRONGBACK COLUMNS PROVIDE SEISMIC ISOLATION JOINT TO AVOID POUNDING S2. OF THE TALLER STRUCTURE INTO THE LOWER STRUCTURE. PROVIDE ALL NEW GRAVITY FRAMING AND LATERAL RESISTING ELEMENTS AS NECESSARY TO PROVIDE
 - BUILDING SEPARATION. A. PROVIDE NEW BEAM CONNECTIONS AND LEDGERS THAT CAN ACCOMMODATE THE REQUIRED DIFFERENTIAL OUT-OF-PLANE MOVEMENT WHILE TRANSFERRING GRAVITY AND IN-PLANE LATERAL
 - FORCES AS NEEDED. В. PROVIDE SEISMIC ISOLATION JOINT TO AVOID POUNDING OF THE STRUCTURES. PROVIDE ALL NEW GRAVITY FRAMING AND LATERAL RESISTING ELEMENTS AS NECESSARY TO PROVIDE BUILDING SEPARATION
- PROVIDE AN INDEPENDENT BRACING SYSTEM OR ANCHOR S3. THE MEZZANINE TO THE SEISMIC-FORCE-RESISTING ELEMENTS OF THE MAIN STRUCTURE. S4.
 - PROVIDE NEW VERTICAL LATERAL RESISTING ELEMENTS. A. INSTALL NEW REINFORCED CMU WALL AT WINDOW LOCATIONS.
 - B. INSTALL NEW PLYWOOD SHEATHING AT EXISTING WOOD FRAMED WALL.
- INSTALL NEW OUT-OF-PLANE ANCHORAGE. S6.

S5.

S7.

- INSTALL NEW OUT-OF-PLANE ANCHORAGE. INSTALL NEW HARDWARE FOR TRANSFER OF SEISMIC FORCES FROM DIAPHRAGM TO SHEAR WALLS.
- 59
- S10
- S11
- PROVIDE NEW DRAG ELEMENTS. INSTALL NEW PLYWOOD DIAPHRAGM SHEATHING. INSTALL NEW PLYWOOD DIAPHRAGM SHEATHING. INSTALL NEW BLOCKED PLYWOOD DIAPHRAGM. INSTALL NEW BLOCKED PLYWOOD DIAPHRAGM. INSTALL NEW OUT-OF-PLANE ANCHORAGE. INSTALL NEW SECONDARY SUPPORTS FOR VERTICAL LOAD S12. S13.
- CARRYING FRAMING ELEMENTS. S14 STRENGTHEN (E) GLULAM ARCHES

NON-STRUCTURAL REPAIRS:

- N1. INSTALL FLEXIBLE COUPLINGS FOR FIRE SUPPRESSION PIPING IN ACCORDANCE WITH NFPA-13. BRACE PIPING OR DUCTWORK CONVEYING N2.
- HAZARDOUS MATERIALS. N3.
- INSTALL SHUT OFF VALVES FOR PIPING CONTAINING HAZARDOUS MATERIAL, INCLUDING NATURAL GAS.
- INSTALL FLEXIBLE COUPLINGS FOR DUCTWORK N4. AND PIPING CONTAINING HAZARDOUS MATERIAL, INCLUDING NATURAL GAS PIPING. INSTALL SEISMIC JOINT COUPLINGS FOR PIPING
- N5. OR DUCTWORK CARRYING HAZARDOUS MATERIAL.
- BRACE UNREINFORCED MASONRY OR N6. HOLLOW-CLAY TILE PARTITIONS. INSTALL SAFETY DEVICES FOR LIGHT FIXTURE N7.
- LENS COVERS. N8. PROVIDE BRACING AND ANCHORAGE OF
- STORAGE RACKS.
- ANCHOR CONTENTS TO THE STRUCTURE. N9.
- N10. BRACE EQUIPMENT TO STRUCTURE.
- BRACE AND ANCHOR EQUIPMENT WEIGHING N11. MORE THAN 20 LB, WHOSE CENTER OF MASS IS MORE THAN 4 FT ABOVE THE ADJACENT FLOOR LEVEL.
- INDEPENDENTLY SUPPORT AND LATERALLY N12. BRACE EQUIPMENT WITH AN OPERATING WEIGHT MORE THAN 75 LB INSTALLED IN LINE WITH A DUCT OR PIPING SYSTEM.
- INSTALL FLEXIBLE COUPLINGS FOR FLUID AND N13. GAS PIPING.
- ANCHOR AND BRACE FLUID AND GAS PIPING TO N14.
- THE STRUCTURE. INSTALL COUPLINGS FOR PIPING THAT CROSSES N15. SEISMIC JOINTS OR ISOLATION PLANES OR IS CONNECTED TO INDEPENDENT STRUCTURES.



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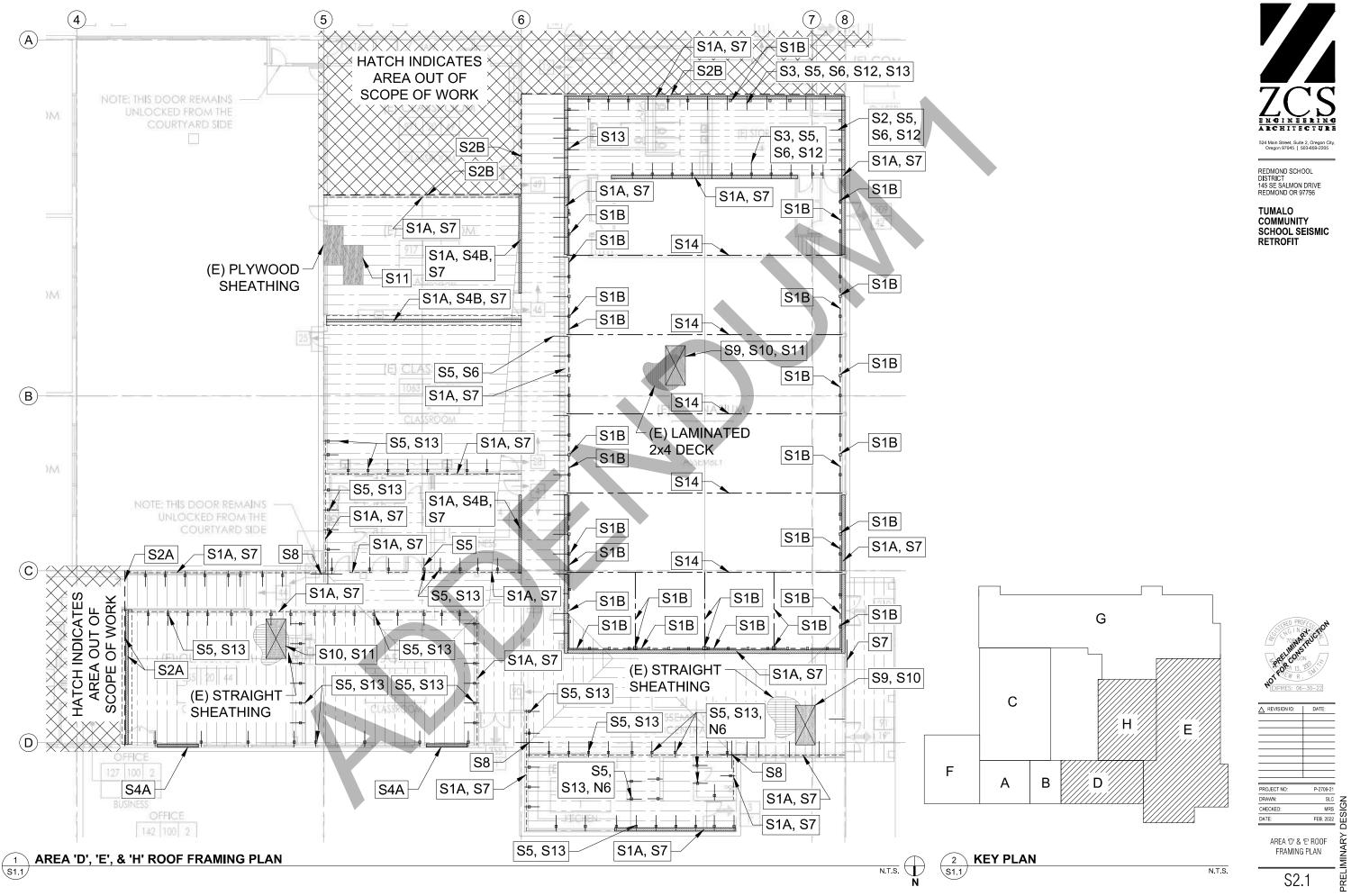
REDMOND SCHOOL DISTRICT 145 SE SALMON DRIVE REDMOND OR 97756

TUMALO COMMUNITY SCHOOL SEISMIC RETROFIT



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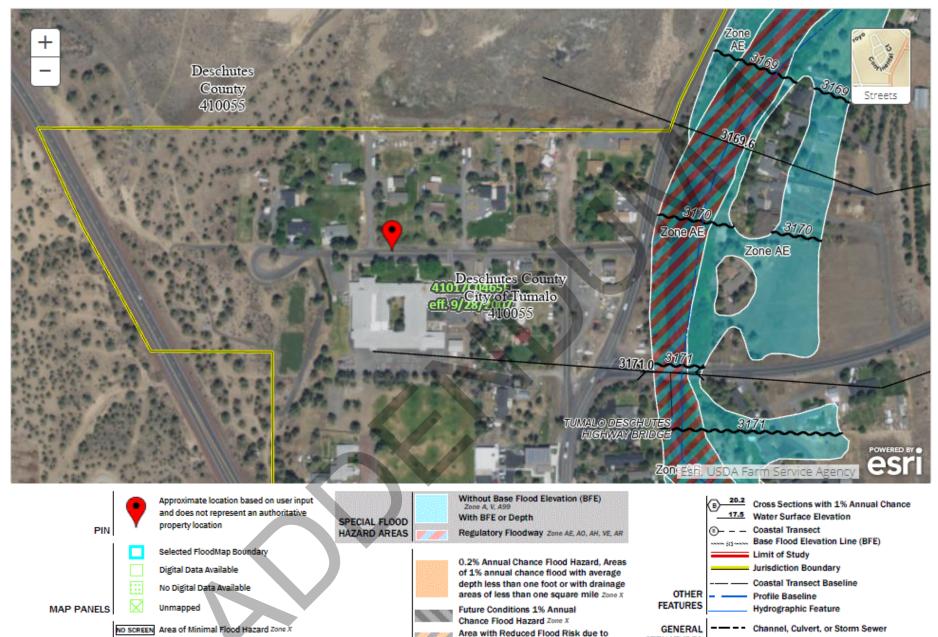


Redmond School District Tumalo Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix D: Geotechnical Information



23



Levee, See Notes, Zone X

Area with Flood Risk due to Levee Zone D

STRUCTURES IIIIII Levee, Dike, or Floodwall

Area of Undetermined Flood Hazard Zone D FLOOD HAZARD

OTHER AREAS OF

A A

Otherwise Protected Area

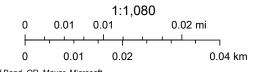
OTHER AREAS

Effective LOMRs

Tumalo ES Liquefaction Map







City of Bend, OR, Maxar, Microsoft

Tumalo ES Landslide Map



December 16, 2022 Landslide Hazard

- Low Landsliding Unlikely
- Moderate Landsliding Possible
- High Landsliding Likely
 - Very High Existing Landslide

1:2,257 0 0.02 0.04 0.08 mi 0 0.03 0.06 0.12 km

City of Bend, OR, Maxar



OSHPD

Tumalo Community School

19835 2nd St, Bend, OR 97701, USA

Latitude, Longitude: 44.1512948, -121.3328812

Google	Tumalo Commu School (Elementa 2nd St	2nd St	Laidlaw Lh Map data ©2021
Date		11/3/2021, 5:00:26 PM	
Design Code Reference Documer	nt	ASCE41-17	
Custom Probability Site Class		D - Default (See Section 11.4.3)	
Type Hazard Level	Description		Value BSE-2N
S _S	spectral response (0.2 s)		0.386
S ₁	spectral response (1.0 s)		0.2
			0.575
S _{XS}	site-modified spectral response (0.2 s)		
S _{X1}	site-modified spectral response (1.0 s)		0.44
Fa	site amplification factor (0.2 s)		1.491
F _v	site amplification factor (1.0 s)		2.201
ssuh	max direction uniform hazard (0.2 s)		0.426
crs	coefficient of risk (0.2 s)		0.905
ssrt	risk-targeted hazard (0.2 s)		0.386
ssd	deterministic hazard (0.2 s)		1.5
s1uh	max direction uniform hazard (1.0 s)		0.227
cr1	coefficient of risk (1.0 s)		0.881
s1rt	risk-targeted hazard (1.0 s)		0.2
s1d	deterministic hazard (1.0 s)		0.6
Туре	Description		Value
Hazard Level	Description		BSE-1N
S _{XS}	site-modified spectral response (0.2 s)		0.384
S _{X1}	site-modified spectral response (1.0 s)		0.293
~x1			0.293

Туре	Description	Value
Hazard Level		BSE-2E
SS	spectral response (0.2 s)	0.262
S ₁	spectral response (1.0 s)	0.136
S _{XS}	site-modified spectral response (0.2 s)	0.417
S _{X1}	site-modified spectral response (1.0 s)	0.318
f _a	site amplification factor (0.2 s)	1.59
f _v	site amplification factor (1.0 s)	2.327

Туре	Description	Value
Hazard Level		BSE-1E
S _S	spectral response (0.2 s)	0.103
S ₁	spectral response (1.0 s)	0.047
S _{XS}	site-modified spectral response (0.2 s)	0.164
S _{X1}	site-modified spectral response (1.0 s)	0.113
F _a	site amplification factor (0.2 s)	1.6
F _v	site amplification factor (1.0 s)	2.4
Туре	Description	Value
Hazard Level		TL Data
T-Sub-L	Long-period transition period in seconds	16

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GEOTECHNICAL INVESTIGATION

PROPOSED ADDITION TUMALO ELEMENTARY SCHOOL 19835 SECOND STREET TUMALO, OREGON 97701

MARCH 1994

PREPARED FOR:

GARY GROFF MORRISON KNUDSON C/O REDMOND SCHOOL DISTRICT 2J 716 SW EVERGREEN AVENUE REDMOND, OREGON 97756

PREPARED BY:

CENTURY WEST ENGINEERING CORPORATION 1444 NW COLLEGE WAY BEND, OREGON 97701

Project No: 11053.002.05

Redmond School District 2J

Geotechnical Investigation - Tumalo Elementary School Proposed Addition

The findings of this report are valid as of the present date; however, changes in the condition of a property can occur with the passage of time, whether they be due to natural process, or the works of man on this or adjacent properties. In addition, changes in applicable or appropriate standards may occur in the future from legislation and the broadening of knowledge.

Accordingly, the findings of this report may be invalidated, wholly or partially, by changes outside of our control. These opinions have been derived in accordance with the current standard of practice and no warranty is expressed or implied.

If you have any questions concerning this report or the exploration, do not hesitate to contact our office.

Sincerely

Glenn E. Cook, P.E. Geotechnical Engineer

OREGON



7

TABLE OF CONTENTS

INTRODUCTION	1
SUMMARY AND CONCLUSIONS	1
PROPOSED CONSTRUCTION	2
EXISTING SITE CONDITIONS	2
FIELD EXPLORATION	2
LABORATORY TESTING	3
SUBSURFACE CONDITIONS	3
FOUNDATION RECOMMENDATIONS	3
FLOOR SLABS	4
DRAINAGE	5
SITE GRADING AND EXCAVATION	5
PAVEMENT RECOMMENDATIONS	6
LIMITATIONS	6

FIGURES

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VICINITY MAP	gure 1
LOCATION OF TEST PITS AND BORING HOLES	gure 2
PROFILES AND LOGS OF EXPLORATORY BORINGS	res 3-5
CROSS SECTIONS OF EXPLORATORY BORINGS	gure 6

GEOTECHNICAL INVESTIGATION PROPOSED ADDITION - TUMALO ELEMENTARY SCHOOL 19835 SECOND STREET TUMALO, OREGON 97701

INTRODUCTION

This report presents results of a Geotechnical Investigation at the site of the proposed addition to the Tumalo Elementary School in Tumalo, Oregon. The facility is located at 19835 Second Street as shown on the Vicinity Map (Figure 1). The majority of the addition will be located at the South end of the existing school. The purpose of the investigation is to provide information and design guidelines for foundation systems, drainage, site grading, excavation, and pavement recommendations.

Data obtained during the field exploration is summarized in Figures 3-6. The recommendations, conclusions, and opinions presented in this report are based on field data obtained, and on our experience with similar projects.

SUMMARY AND CONCLUSIONS

- 1. Sub-surface conditions were explored by drilling five air-track borings and excavating three backhoe test pits on November 11, 1993.
- 2. The proposed addition should be founded on conventional spread footings bearing on recompacted native sand material. Recompaction requirements and design criteria for spread footings are given in this report. Boring cross sections within the building areas are shown in Figure 6.
- 3. The on-site sand overburden can be used as structural fill. If import material is required for development of the project a granular material is recommended.
- 4. The sand overburden can be excavated with conventional excavation equipment. If excavation of the underlying hard basalt is required, blasting and/or chipping with hydraulic hammers is anticipated.

1



PROPOSED CONSTRUCTION

As presently planned, the addition will be a single story structure consisting of a new cafeteria/multiuse building, a new library, and six new classrooms. The cafeteria/multi-use building will be located at the Northwest corner of the school and the library and the classroom will be located at the South end of the school. Total square footage of the new addition will be approximately 12,400 square feet. Approximate wall loads for this type of structure would be 2.0 kips per linear foot. It appears that the existing septic system located South of the proposed library will require relocation.

EXISTING SITE CONDITIONS

The site for the proposed library and classrooms is partially occupied by an asphalt play area, lawn, and miscellaneous landscape. An in place septic system is located along the South wall of the proposed library.

The site for the proposed cafeteria/multi-use building is occupied by lawn and landscape area. A concrete septic tank cover is located along the North wall of the addition. It appears that the in place septic systems will require relocation. The site topography map also indicates underground sewer lines in the area of the proposed addition.

FIELD EXPLORATION

Subsurface conditions were explored by performing five air-track boring and three test pits at the subject site. The boring and pit locations are shown in Figure 2. The air-track drilling method provides a profile of the rock, but does not provide engineering information on the overburden soils. The air-track logs and penetration rates are shown in Figures 3 and 4.

Air-track borings indicate overburden soil material of over five feet. Because of the soil depths observed, additional exploration was conducted using backhoe test pits. The test pits were excavated with a John Deere 410 backhoe to obtain adequate engineering information of the soil materials.



At intervals of approximately two feet, in-place density tests were taken using a Troxler moisturedensity gauge. Disturbed bag samples were also taken within the soil profile.

LABORATORY TESTING

Samples were returned to the laboratory where they were visually classified. Soil classification tests, including gradation and moisture content, were conducted on representative samples of the various soil layers. Standard proctor testing was also conducted. The results of the laboratory testing are presented in Figure 5.

SUBSURFACE CONDITIONS

The air-track borings indicate five to six feet of sand overburden overlying river cobbles with a sand and gravel matrix and a moderately hard basalt rock. The basalt rock is slightly fractures and has a variable penetration rate. Occasionally, the penetration rates change abruptly, indicating a possible contact zone of two successive lava flows. In a few instances the drill rod would penetrate rapidly for several inches. This generally indicates a thin void, vertical crevice, or possible soil filled void. In general, penetration rates of less than 10 seconds/inch would be characteristic of a void, crevice, or soil layer. Rates of 10 to 30 seconds/inch indicate rock which is moderately massive with closely spaced fractures and thin void, crevices, or soil layers.

The backhoe test pits excavated at the South and West end of the existing building confirmed the basalt rock depth of approximately six feet below existing surface. The sand overburden is dry to moist in moisture content, and medium dense in consistency The in-place dry density of the sand overburden at approximate proposed footing depth varied from 80.0 to 87.7 pounds per cubic foot (pcf), which equates to a relative compaction of 84.6 to 92.8 percent of standard proctor.

FOUNDATION RECOMMENDATIONS

The proposed addition will use a slab-on-grade foundation system with conventional spread footings. Floor elevation will generally match the floor elevation of the adjacent existing structures.



The footings should be designed for an allowable soil bearing pressure not to exceed 2,500 pounds per square foot for dead load plus live load. Due to the variable in place relative densities at proposed footing depths, the bottom of all footing trenches should be compacted to 95% of standard proctor. The exposed footing bottom should be compacted using a small self propelled smooth drum vibrating roller, or a hoe-pack. All exterior foundations should bear below the frost depth. The following design criteria should be observed:

- a) Frost depth for the Redmond area is estimated at 24 inches.
- b) All footing should have a minimum width of 18 inches.
- c) Allowable soil bearing pressure may be increased one-third for wind and/or seismic condition.
- d) Maximum settlement should not exceed one inch under full dead load plus live load conditions. In general, differential settlement should be less than 1/2 inch. Most settlement will occur during construction, about seven days after loads are initially applied.
- e) We recommend that continuous concrete foundation walls be reinforced to span a distance of six feet in length. This should reduce the potential effect of differential settlement.
- f) In general, all vegetation, asphalt paving, concrete walks, and the upper four inches of organic top soil (lawn area) should be stripped off the building area. After the site has been cleared, the footing trenches excavated and the footing trenches compacted the site should be observed by the Geotechnical Engineer. The purpose of this visit is to confirm the recommended bearing pressures and to verify that sufficient organics have been removed.

FLOOR SLABS

It is our understanding that the structure will have a slab-on-grade floor system. Depending on finish slab elevations, structural fill may be required. The vegetation, asphalt paving, concrete walks, and the upper 4 inches of organic top soil (lawn area) should be stripped from the building area.



The exposed soils should be moistened and compacted prior to placement of fill. The fill should be placed in 8 inch loose layers and compacted with heavy compaction equipment to 95 percent of standard proctor density ASTM D698. In the building area, a six inch layer of clean underslab crushed gravel is recommended. The crushed gravel should be compacted to 95 percent of standard proctor ASTM D698.

DRAINAGE

The site topography map (Figure 2) indicates the area of the proposed addition is relatively flat. If pavement and/or surface grades in the building area slope toward the building, it is recommended that a foundation sub-drain and visqueen moisture barrier be installed. If the building level is high enough to allow positive drainage away from the building, and water is not allowed to pond next to the building, then a foundation drain and moisture barrier are probably not warranted. A minimum surface grade of three percent is recommended.

SITE GRADING AND EXCAVATIONS

At the time of this writing, final grading plans were not available. It is assumed that minimal site grading will be required in development of the new addition. If fill is required around the proposed structure, it is recommended that the top 4 inches of top soil (lawn area) be removed and that the existing asphalt paving and concrete walk be removed. Tree stumps and root systems should be removed. After removals have been completed, the exposed silty sand should be scarified 8 inches and compacted with heavy compaction equipment. The exposed soils and structural fill should be compacted to 95 percent of standard proctor.

The native sand material can be used as structural fill. Import fill, if required, should be granular in nature and be approved by the Geotechnical Engineer prior to hauling to the site. In general, acceptable fill material would be well graded with less than ten percent passing a No. 200 sieve with maximum particle size of one inch. Placement of structural fill should be observed and tested by a qualified technician working under the direction of the Geotechnical Engineer. A suitable testing frequency would be to test every one foot of fill depth as it is placed.

The sand overburden can be excavated with conventional excavation equipment. If excavation is required in the basalt rock (six feet below existing surface), blasting and/or chipping with a hydraulic hammer is anticipated.



PAVEMENT RECOMMENDATIONS

The shallow sub-grade soils will probably consist of the overburden silty sand material which will provide good pavement support. Most of the vehicular traffic is expected to be automobile or light trucks. We recommend a pavement section of 2.5 inches of Asphaltic Concrete Surface (ACS) underlain by four inches of crushed Aggregate Base Course (ABC) for automobile parking areas. If bus lanes and/or service driveways are required, we recommend a pavement section of three inches of ACS overlying 8 inches of ABC. The ACS should be compacted to 90 percent of Marshall density (ASTM D1559). The asphalt aggregate should meet the requirements for Class B or C, per Oregon Department of Transportation (ODOT) specifications. The ABC should consist of a 3/4-inch minus crushed aggregate. The subgrade and ABC should be compacted to 95 percent of standard Proctor density, ASTM D698.

LIMITATIONS

The recommendations of this report pertain only to the site investigated and are based upon the assumption that the soil conditions do not deviate substantially from those encountered or indicated during this investigation. If significant variations or undesirable conditions are encountered during construction, or if the proposed construction will differ from the construction planned at the present time, Century West Engineering Corporation should be notified so that supplemental recommendations can be given.

This report is issued with the understanding that it is the responsibility of the Owner, or of his representative, to insure that the information and recommendations contained herein are brought to the attention of the Structural Engineer for the project and incorporated into the plans, and that the necessary steps are taken by the Owner to carry out such recommendations in the field. Placement and compaction of fill and construction materials should be observed and tested by a certified materials testing lab working under the direction of a Geotechnical Engineer.



The findings of this report are valid as of the present date; however, changes in the condition of a property can occur with the passage of time, whether they be due to natural process, or the works of man on this or adjacent properties. In addition, changes in applicable or appropriate standards may occur in the future from legislation and the broadening of knowledge.

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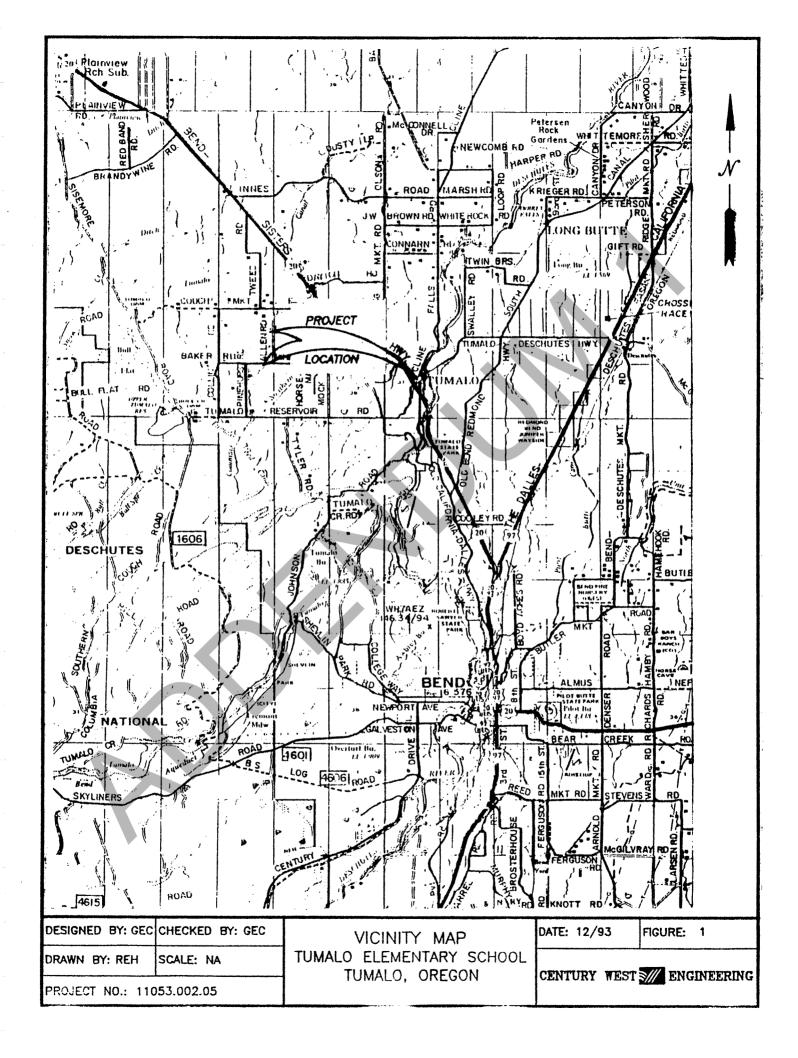
Sincerely

Glenn E. Cook, P.E. Geotechnical Engineer

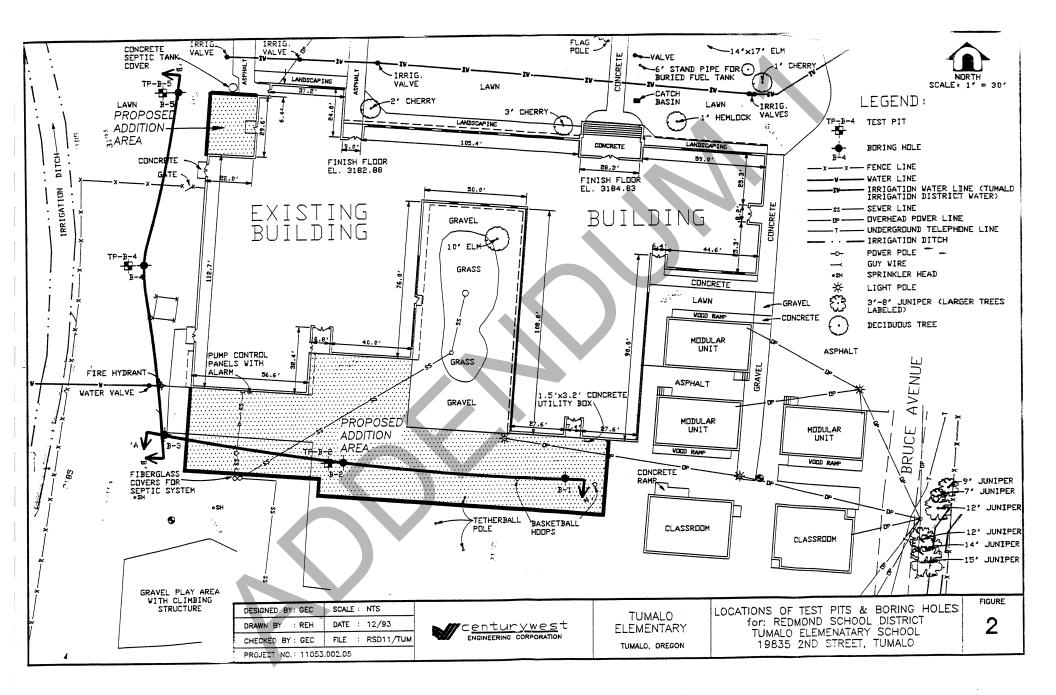




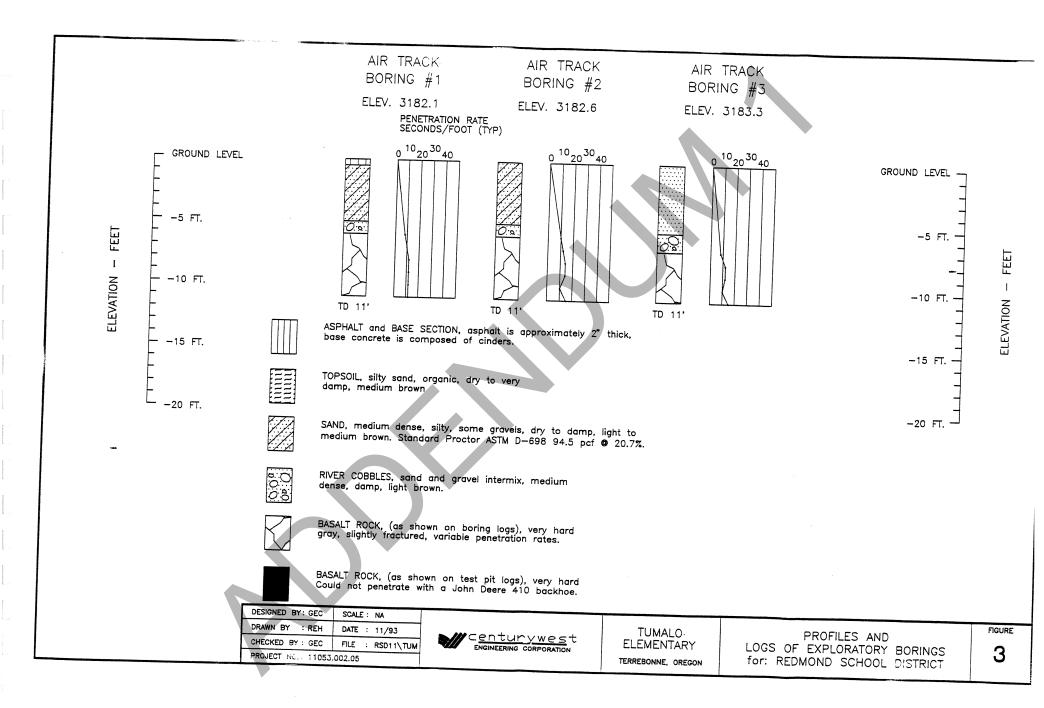




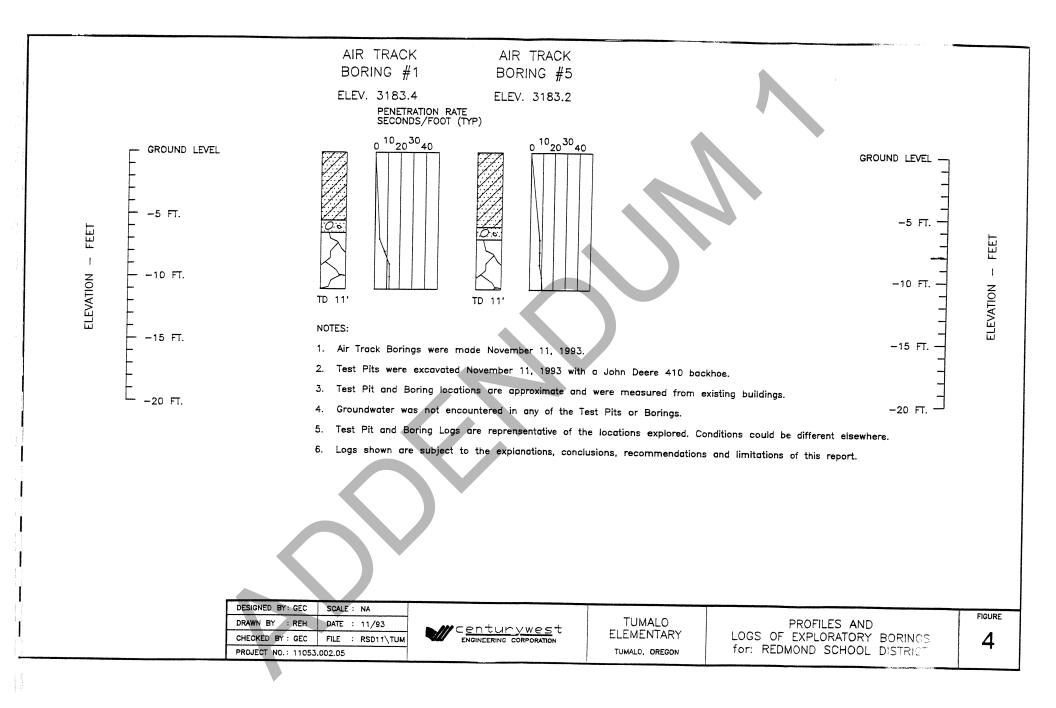




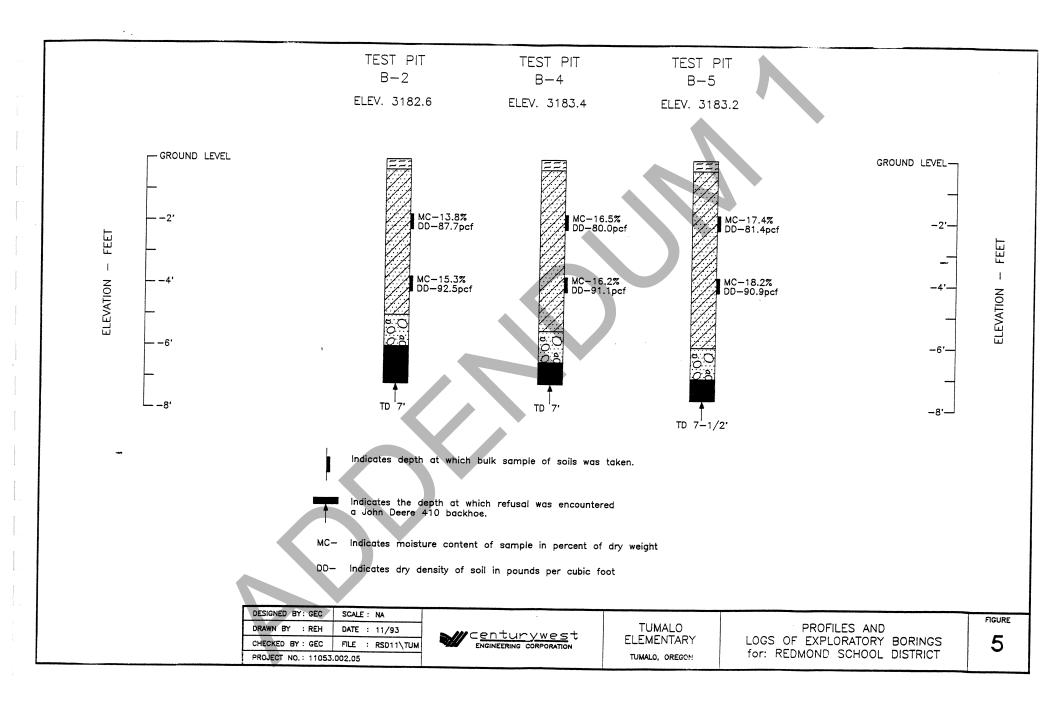




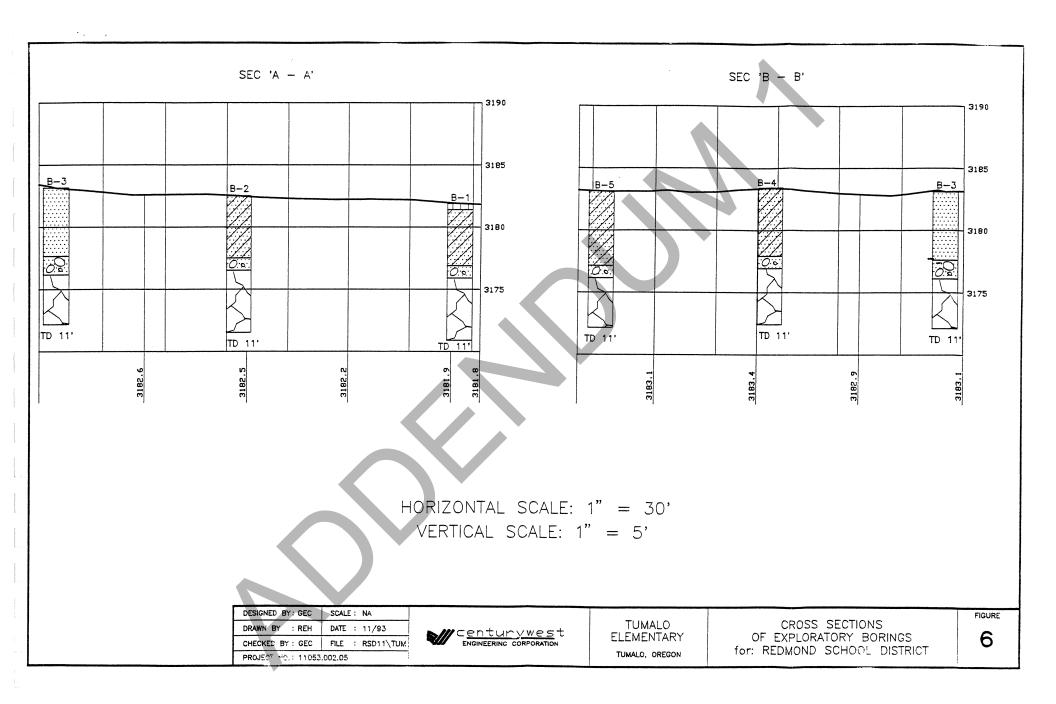














02-6101-01 February 28, 2022

Sy Allen, Principal Engineer ZCS Engineering & Architecture 900 Klamath Avenue Klamath Falls, Oregon 97601

^C/o Stephen Chase, Lead Designer ZCS Engineering & Architecture 127 NW D Street Grants Pass, Oregon 97526

SUBJECT: SEISMIC HAZARDS REVIEW TUMALO COMMUNITY SCHOOL 19835 2nd STREET TUMALO, OREGON

Mr. Chase:

This report presents the results of our preliminary review and evaluation of the Tumalo Community School for a potential Seismic Retrofit of the existing school structures. The subject school is located at 19835 2nd Street, in Tumalo, Oregon.

The purpose of this memo report was to conduct a planning level review and seismic risk assessment (office studies) in order to provide preliminary geologic information and evaluate the likelihood and consequences of geotechnical/geologic related seismic failures, including liquefaction and landslide potential during the design seismic event, for consideration regarding the potential seismic retrofit.

SITE AND PROJECT DESCRIPTION

The site is currently occupied by a functioning elementary school. The school facilities currently consist of a complex of multiple structures with direct/adjacent connections. The school complex is surrounded by lawn/landscaping areas, access roads, parking lots, walkways, play fields and open space. The site is relatively flat and the undeveloped portions of the site consist of well-maintained lawn and scattered trees.

We understand the ZCS Engineering and Architecture consulting design team is conducting a preliminary facilities review to determine the level and extent of seismic retrofit needed for the structures on this campus. Their review will be based, in part, on the evaluation of the potential geologic hazards (such as liquefaction) provided in this report, and an evaluation of the potential structural damage to these facilities associated with the design seismic event.

This evaluation and the findings and conclusions of the facilities review will also likely be used to pursue grant funding for completion of the seismic retrofit work.

SITE SOIL AND WATER CONDITIONS

The site subsurface soils and water conditions were reviewed based on information provided in a previous geotechnical investigation accomplished by Century West Engineering Corporation at this site (accomplished in March 1994 as part of the large addition to southern end of the school structure). We also reviewed the available nearby water well and geotechnical boring logs (Oregon Water Resources Department website).

Soils. From our review, it appears the upper subsurface soils across the site are relatively uniform across the site. The surficial soils in the upper 5 to 6 feet beneath the surface consist of medium dense to dense silty, Sands with scattered gravels. This is underlain by a 2' to 3' layer of medium dense to dense sandy Gravels and Cobbles. Practical refusal in the test pits, accomplished using a John Deere 410 backhoe, was encountered at depths ranging 7' to 7.5' below the surface at the top of the underlying fractured basalt rock. All of the Test Pits and Borings accomplished on the subject parcel terminated in the very dense, stable unit of fractured basalt rock.

Groundwater. Groundwater was not encountered in any of the Test Pits or Borings during Century West's geotechnical investigation. We do not anticipate the water table getting close to the surface, given the subsurface conditions encountered and underlying fractured basalt rock. Ground water will likely not be an issue on this site during construction of the project. Regional groundwater levels will be 100 feet or deeper. However, due to the shallow, dense, weathered to fractured rock it would appear that during very wet months there could be small amounts of seepage of perched water on top of the underlying rock.

Please note that the soils and water conditions are described as distinct layers, while in nature they may change more gradually. Soils conditions may also change somewhat at other locations across the project site.

SITE GEOLOGY AND SEISMIC INDUCED HAZARDS REVIEW

Summary of Site Geology. Mapped geologic units in the project area consist primarily of Alluvial Fan deposits and volcanic bedrock members of the Deschutes Bend Tuffs Formations (Sherrod, et al., 2004). Beneath the surficial Sand and Gravel/Cobble soils, the mapped bedrock unit at the project site consists of the deeply embedded volcanic rocks comprised of basaltic andesite and volcaniclastic ashflow tuff. Based on the site subsurface information provided by Century West, the bedrock encountered on the subject site was described as basalt bedrock.

Flooding. The site is not within a 100-year floodplain of any river or streams according to FEMA and Oregon HazVu mapping.

Landslides/Slope Instability. The project site is relatively flat and is not located within a mapped Quaternary landside area (Qls), based on our review of the state landslide database (Statewide Landslide Information Database for Oregon; SLIDO, 2017) and aerial photos

(Google Earth, 2020), as well as from the subsurface data obtained from Century West's subsurface investigation. Therefore, possibility of slope failure, rock fall or slide run out damage at the site is considered low.

Liquefaction and Lateral Spread Hazard Potential. The project is underlain by medium dense to dense Sands and sandy Gravels and Cobbles. Soils with densities similar to the conditions indicated in the Test Pit and Boring exploration have not been known to liquefy in a seismic event. In addition, groundwater levels appear to be over 100 feet below the ground surface based on nearby well log data. <u>Therefore, liquefaction and lateral spread is</u> <u>considered to be a low to very low potential hazard for this site.</u> See more information in the Preliminary Liquefaction Evaluation section of this report.

Ground Rupture. No large Quaternary faults were identified at the project site. However, mapped fault lines of the Sister Fault Zone are located approximately 0.5 to 1.0 mile from the project. Therefore, the risk of damage at the site due to ground rupture is considered low.

Ground Shaking. Project structures, including foundations and retaining walls, must be designed for very severe ground shaking potential during the anticipated seismic event. The peak modified horizontal acceleration (PGA_M) at this site is 0.253g. This is based on a Site Class D designation, determined for the project from our review of the subsurface Boring and Test Pit data provided by Century West and from our review of nearby well logs. This PGA_M value may be used with an appropriate seismic coefficient in pseudo static analysis, for existing structures evaluation purposes and for design of the seismic upgrades.

Seismic Ground Amplification or Resonance. No unusually hazardous amplification or resonance effects from seismic waves have been associated with the subsurface soil/bedrock conditions in the project area.

Tsunami and Seiche. The site is approximately 85 miles inland from the coast, and not subject to tsunami hazard. The site is not located adjacent to a large lake or body of water, and therefore, not subject to seiche hazard.

PRELIMINARY LIQUEFACTION EVALUATION

The liquefaction phenomenon occurs in cohesionless soils (non-plastic silts and sands) that are saturated and loose (low density, uncompacted or poorly compacted). When loose cohesionless soils are saturated, which is the case when soil is below the water table, then water fills the soil pores. In response to compression (i.e. when a load is applied to the loose, saturated soil), the increases in pressure on the water causes it to attempt to migrate or dissipate towards zones of low pressure (i.e. the water gets pushed/pumped to portions of the soil where the soil pores are not already filled). It should be noted that water, in a practical sense, is an incompressible liquid (very highly resistant to changes in volume when subjected to changes in pressure). Therefore, if the applied load is rapid and large enough, or if it is repeated many times (cyclic loading) like during an earthquake, such that there is not enough time for the water to dissipate before the next cycle of loading is applied, then the water pressure may build up in the pores to a degree where it becomes greater than the grain-tograin contact stresses of the soil. The grain-to-grain contact stresses are the source of the soil shear strength and stability which supports structures foundations and overburden soils. This buildup of excess pore water pressure can result in a partial or total loss of the soil strength, at which point the soil will lose all its stability, be deformed (may be observed to flow like a liquid, hence "liquefaction"), and will not likely be able to support structures.

Based on our review, the site is underlain by medium dense to dense Sand and Gravels/Cobbles. Groundwater was not encountered in any of the borings or test pits and nearby well logs show that groundwater is at least 100 feet deep. Soils with these densities and in an unsaturated condition are not known to liquefy in a seismic event. <u>Therefore, in our professional opinion, the potential for liquefaction of the medium dense to dense, sandy and gravelly/cobbly soils that could adversely affect the site or have significant adverse impacts on the structures during a seismic event is low.</u>

CONCLUSIONS AND RECOMMENDATIONS

Based on our field investigation and office review, in our professional opinion the soils conditions at the site are suitable for a conventional seismic retrofit. This school site is not susceptible to large scale liquefaction that will adversely impact the structure. However, prior to final design and construction, more detailed geotechnical investigation and laboratory testing will be necessary to provide support/mitigation recommendations.

Given the alluvial nature of the site soils, additional borings around the structures may encounter sandy soils layers. These soils could potentially be liquefiable. However, these are likely to be moderate to small in size/thickness and should not adversely impact the overall site stability or increase the potential damage to the school structures during a seismic event.

If/when the final design and construction phase of work for this seismic retrofit project begins, we anticipate the following additional tasks will need to be accomplished:

- 1. 2 or 3 additional borings.
- 2. Laboratory testing for determining expansive index, strength and settlement characteristics of the site soils.
- 3. Evaluation of data for developing geotechnical design parameters and recommendations (excavations/embedment depths, subgrade preparations, cuts/fills, and foundation/slab support, etc.).
- 4. Ground motion hazard analysis to determine spectral acceleration parameters for the school structures and retrofit elements.

These items would be provided as part of a final Seismic Retrofit Geotechnical Design Report.

LIMITATIONS

The analyses, conclusions and recommendations contained in this report are based on site conditions as they existed at the time of the study, and assume our review of the soils, rock and groundwater conditions specified in the Century West Geotechnical Investigation Report are representative of soils and groundwater conditions throughout the site. If subsurface

conditions or assumed design information is found to be different, we should be advised at once so that we can review this report and reconsider our recommendations in light of the changed conditions. If there is a significant lapse of time (5 years) between submission of this report and the start of work at the site, if the project is changed, or if conditions have changed due to acts of God or construction at or adjacent to the site, it is recommended that this report be reviewed in light of the changed conditions and/or time lapse.

This report was prepared for the use of the ZCS Engineering and Architecture and their design team for evaluation purposes. It should be made available to contractors for information and factual data only. This report should not be used for contractual purposes as a warranty of site subsurface conditions. It should also not be used at other sites or for projects other than the one intended.

We have performed these services in accordance with generally accepted geotechnical engineering and professional geology practices in Oregon, at the time the study was accomplished. No other warranties, either expressed or implied, are provided.

 THE GALLI GROUP GEOTECHNICAL CONSULTING

 Dennis Duru, M.Sc., R.G. Project Geologist

 Multiple

 Melvin J. Galli III, P.E. Senior Principal Engineer

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Bibliography

Sherrod, D., Taylor, E., Ferns, M., Scott, W., Smith, G., & Conrey, R. (2004). *Geologic map* of the Bend 30- x 60- Minute Quadrangle, Central Oregon. USGS.

Redmond School District Tumalo Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix E: Construction Cost Estimate Worksheets

ENGINEER'S OF	PINION OF PROBABLE C	OST - TUMALO COM	MUNITY SCHOOL SEISMI	C REHABILITAT	ION
		SUMMARY			
Description	Deficiencies (Ref. Seismic Evaluation Report Sec. 7.0)	Unit Price	Total Price for Construction Item		
		GENERAL CONDITI	ONS		
General Conditions Preconstruction Services		10% 2%	% %		\$ 137,307.50 \$ 27,461.50
Escalation Bonding & Insurance Contractor Profit & Overhead		7% 3% 5%	% % %		\$ 107,649.08 \$ 46,135.32 \$ 76,892.20
			General C	Conditions Subtotal	\$ 395,445.60
		Non-Structural Elem	ents		
Misc MEP Misc Non-Structural	N1, N2, N3, N4, N5, N11, N12, N13 N14 N15 N7, N8, N9, N10	1 1	Lump Sum Lump Sum	\$ 90,000.00 \$ 36,000.00	\$ 90,000.00 \$ 36,000.00
			Non-	Structural Subtotal	\$ 126,000.00
	Const	truction Cost Per Bu	ilding Part		
				ng Part 'A' Subtotal	\$-
			Buildir	ng Part 'E' Subtotal	\$ 898,000.00
			Buildir	ng Part 'H' Subtotal	\$ 161,075.00
			Buildir	ng Part 'D' Subtotal	\$ 188,000.00
			Sub-Total Cor	struction Cost	\$ 1,768,500.00
			Contingency	15%	\$ 265,275.00
			Total Cor	struction Cost	\$ 2,033,775.00
		Cost Estimate Sum	mary		
Engineering Architectural Consulting Structural / Rehabilitation Engineering Geotechnical Consulting Materials Testing for Design URM Tier 3 Analysis	J J			\$ 30,500.00 \$ 223,700.00 \$ 20,000.00 \$ 10,200.00 \$ 5,000.00	\$ 289,400.00
Construction Management Construction Sub-Total Construction Cost Special Inspection Services for Construction Permitting Fees Relocation of FF&E		\sim		\$ 1,768,500.00 \$ 10,200.00 \$ 61,000.00	\$ 61,000.00 \$ 1,839,700.00 \$ 26,500.00
Contingency			Total Ducio of Furrelline		\$ 265,275.00
			Total Project Funding	Requirement	\$ 2,481,875.00

TPO / Comp / Metal Roof Demo	S2A, S10, S11 S2A, S4A, S5, S6, S13 S2A, S4A S4A	BUILDING PART - Quantity olition & Asbestos A 3000 2000 400 200 7 / Floor Strengthenin 30	Units batement Square Foot Square Foot Square Foot Square Foot Square Foot	Unit Price	Total Price for Construction It \$ 6, \$ 4, \$ 8, \$ 1,
Soft Demolition S1A, Hard Demolition Abatement	Dem S2A, S10, S11 S2A, S4A, S5, S6, S13 S2A, S4A S4A Foundation	3000 2000 400 200	Square Foot Square Foot Square Foot Square Foot Demolition	\$ 2.00 \$ 20.00 \$ 5.00	\$6, \$4, \$8,
Soft Demolition S1A, Hard Demolition Abatement	S2A, S10, S11 S2A, S4A, S5, S6, S13 S2A, S4A S4A Foundation	3000 2000 400 200	Square Foot Square Foot Square Foot Square Foot Demolition	\$ 2.00 \$ 20.00 \$ 5.00	\$4, \$8,
Hard Demolition Abatement	S2A, S4A S4A Foundation	400 200 n / Floor Strengtheniu	Square Foot Square Foot Demolition	\$ 20.00 \$ 5.00	\$ 8,
Abatement	S4A Foundation	200 n / Floor Strengtheniu	Square Foot Demolition	\$ 5.00	
Shear Wall Footings - Wood Walls				& Asbestos Subtotal	
Shear Wall Footings - Wood Walls					\$ 19,0
Shear Wall Footings - Wood Walls			na Construction		
			Linear Foot	\$ 300.00	\$9,
			Foun	dation Level Subtotal	\$ 9,0
	Wal	I Strengthening Con	struction		
	S1A, S4A, S5, S13	600	Square Foot	\$ 2.00	\$1,
-	S1A, S4A, S5, S13	2000 44	Square Foot EA	\$ 3.00	\$6, \$66,
Light Steel Columns New CMU / Concrete Shear Walls	S13 S4A	200	Square Foot	\$ 1,500.00 \$ 30.00	\$ 66, \$ 6,
			Wall St	rengthening Subtotal	\$ 79,2
	Roo	f Strengthening Con	struction		
New Roof Sheathing	S10, S11 S10, S11	3000 3000	Square Foot Square Foot	\$ 4.00 \$ 10.00	
New Composite Roof Shingles Diaphragm Attachments - Out-of-Plane	S5. S6, S12	240	Linear Foot	\$ 10.00 \$ 50.00	\$ 30, \$ 12,
Diaphragm Attachments - In-Plane Shear	S1A, S7	240	Linear Foot	\$ 20.00	\$ 4,
Seismic Isolation from Adjacent Building Ceiling Repair S1A	S2A S2A, S4A, S5, S6, S13	40 2000	Linear Foot Square Foot	\$ 400.00 \$ 3.00	\$ 16, \$ 6,
			Roof St	rengthening Subtotal	\$ 80,8
		В	Building Part 'D' - Total C	onstruction Cost	\$ 188,00

ENGINEER'S	OPINION OF PROBABLE COS	ST - TUMALO COMM	IUNITY SCHOOL SEISMI		ION
		BUILDING PART - 'E	Ξ'		
Description	Deficiencies (Ref. Seismic Evaluation Report Sec. 7.0)	Quantity	Units	Unit Price	Total Price for Construction Item
	Demol	ition & Asbestos Ab	atement		
Abatement TPO / Comp / Metal Roof Demo Hard Demolition Soft Demolition	S1A, S1B, S2A, S3, S5, S6, S7, S12 S9, S10, S11 S1B, S13 S1B	4400 9000 200 3000	Square Foot Square Foot Square Foot Square Foot	\$ 5.00 \$ 2.00 \$ 20.00 \$ 2.00	\$ 22,000.0 \$ 18,000.0 \$ 4,000.0 \$ 6,000.0
			Demolition &	& Asbestos Subtotal	\$ 50,000.00
	Foundation /	Floor Strengthening	g Construction		
Flooring Protection Concrete Repair & Patching Bolting of Extg Walls to footings Spread Footings for Columns / Holdown	S14 S1B, S13 S1A, S4B S1B, S13	5500 1600 300 3	Square Foot Square Foot Linear Foot Each	\$ 6.00 \$ 15.00 \$ 35.00 \$ 4,000.00	\$ 24,000.00 \$ 10,500.00
	•		Found	ation Level Subtotal	\$ 79,500.00
	Wall S	Strengthening Const	ruction		
Sheathing of Existing Walls Interior Wall Finish Repair Painting Steel Spandrel Light Steel Columns Heavy Steel Columns	S1A, S4B S1A, S4B S1A, S4B S1B S1B S1B	3000 3000 6000 250 103 3	Square Foot Square Foot Square Foot Linear Foot EA EA	\$ 5.00 \$ 2.00 \$ 3.00 \$ 600.00 \$ 1,500.00 \$ 7,500.00 \$	\$ 150,000.00 \$ 154,500.00
				engthening Subtotal	\$ 366,000.00
		Strengthening Const			
New Roof Sheathing Diaphragm Attachments - In-Plane Shear Existing Beam Strengthening New 6" polyisociurinate rigid insulation New Composite Roof Shingles Seismic Isolation from Adjacent Building Diaphragm Attachments - Out-of-Plane Ceiling Repair New Drag Beam Attachments New Wood Beams	\$9, \$10, \$11 \$1A, \$7 \$14 \$9, \$10, \$11 \$9, \$10, \$11 \$2B \$3, \$5, \$6, \$12 \$1A, \$1B, \$2A, \$3, \$5, \$6, \$7, \$12 \$7, \$8 \$1B, \$8	9000 400 5 6400 9000 130 3000 3000 8 50	Square Foot Linear Foot EA Square Foot Square Foot Linear Foot Linear Foot Square Foot EA Linear Foot	\$ 4.00 20.00 15,000.00 15,000.00 15,000 15,000 10,00 1	\$ 15,000.00 \$ 9,000.00
				engthening Subtotal	
		B	uilding Part 'E' - Total Co	nstruction Cost	\$ 898,000.00

ENGINEER'S OF	PINION OF PROBABLE CO	OST - TUMALO COM	MUNITY SCHOOL SEISI		ION
		BUILDING PART -	Ή'		
Description	Deficiencies (Ref. Seismic Evaluation Report Sec. 4.0)	Quantity	Units	Unit Price	Total Price for Construction Item
	Demo	olition & Asbestos A			
Abatement TPO / Comp / Metal Roof Demo Hard Demolition	S1A, S4B, S5, S6, S7, S12 S11 S4B	1800 4000 200	Square Foot Square Foot Square Foot		\$ 9,000.00 \$ 8,000.00 \$ 4,000.00
Soft Demolition	S1A, S4B, S5, S6, S7, S12	1800	Square Foot	\$ 2.00	\$ 3,600.00
				& Asbestos Subtotal	\$ 24,600.00
		/ Floor Strengthenir	÷		
Shear Wall Footings - Wood Walls Concrete Repair & Patching	S4B S4B	50 200	Linear Foot Square Foot		\$ 15,000.0 \$ 3,000.0
Bolting of Extg Walls to footings Floor Finish Patch / Replacement	S4B S4B	75 200	Linear Foot Square Foot		\$ 2,625.0 \$ 1,400.0
					*
				dation Level Subtotal	\$ 22,025.00
		Strengthening Cons			
Sheathing of Existing Walls Interior Wall Finish Repair Painting Light Steel Columns	S4B S4B S4B S5, S6, S12, S13	750 1500 1500 22	Square Foot Square Foot Square Foot EA	\$ 2.00 \$ 3.00	\$ 3,750.00 \$ 3,000.00 \$ 4,500.00 \$ 33,000.00
				•	
				rengthening Subtotal	\$ 44,250.00
	Roo	f Strengthening Con	struction		
Re-Nail Existing Plywood New Composite Roof Shingles Diaphragm Attachments - In-Plane Shear Diaphragm Attachments - Out-of-Plane Ceiling Repair New Drag Beam Attachments	S11 S11 S1A, S7 S5, S6, S12 S1A, S4B, S5, S6, S7, S12 S8	4000 4000 260 100 1000 2	Square Foot Square Foot Linear Foot Linear Foot Square Foot EA	\$ 10.00 \$ 20.00 \$ 50.00 \$ 3.00	\$ 12,000.0 \$ 40,000.0 \$ 5,200.0 \$ 5,000.0 \$ 3,000.0 \$ 5,000.0
		_	Roof Si uilding Part 'H' - Total C	• •	\$ 70,200.00 \$ 161,075.00

Redmond School District Tumalo Elementary School Seismic Evaluation February 2022 Project No: P-2706-21

Appendix F: Rapid Visual Screening

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Severe Vertical Irregularity, V_{L1}		-1.3	-1.3	-1.3	-1.1	-1.0	-1.2	-1.0	-0.9	-1.0	-1.1	-0.8	-1.0	-0.9	-1.0	-1.0	-0.8	NA
Moderate Vertical Irregularity, V _L Plan Irregularity, P _{L1}	1	-0.8 -1.3	-0.8 -1.2	-0.8 -1.1	-0.7 -0.9	-0.6 -0.8	-0.8 -1.0	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.9	-0.5 -0.6	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.7	-0.5 -0.5	NA NA
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Post-Benchmark		1.5	1.9	2.3	1.4	1.4	1.0	1.9	NA	1.9	2.1	NA	2.1	2.4	2.1	2.1	NA	1.2
Soil Type A or B		0.3	0.6	0.9	0.6	0.9	0.3	0.9	0.9	0.6	0.8	0.7	0.9	0.7	0.8	0.8	0.6	0.9
Soil Type E (1-3 stories) Soil Type E (> 3 stories)		0.0 -0.5	-0.1 -0.8	-0.3 -1.2	-0.4 -0.7	-0.5 -0.7	0.0 NA	-0.4 -0.7	-0.5 -0.6	-0.2 -0.6	-0.2 -0.8	-0.4 -0.4	-0.5 NA	-0.3 -0.5	-0.4 -0.6	-0.4 -0.7	-0.3 -0.3	-0.5 NA
Minimum Score, S _{MIN}		1.6	1.2	0.8	0.5	0.5	0.9	0.5	0.5	0.3	0.3	0.3	0.3	0.2	0.3	0.3	0.2	1.4
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Y				s. creener(s):				3	1 Da	ate/Time	e:			
rain and a state of the state o			and the second division of the second divisio	o. Stories:				Below	Grade	:	Year	Built:		EST
			Тс	otal Floor A	rea (so	Į. ft.):		-				Year:		
				dditions:			Yes, Ye							
A CALL AND A	and the second		00	ccupancy:	Asse Indu	embly strial	Commerce Office		Emer. Se School	ervices		storic overnmer	Shelte	er
		Carl San			Utilit		Warehou			tial, <i>#</i> Ur		Jverniner	it.	
	Ser all		Sc	oil Type:		B	□C]E []F DI	NK		
	Aunt				Hard Rock	Avg Rock	Dense Soil	e Stif Soi			oor <i>If</i> Soil	DNK, ass	ume Type I	D.
2nd St 2nd St	2nd St		G	eologic Ha								Surf P	int · Yee/N	I0/DNK
	Carlos and	34		djacency:			ounding						t Building	
Tunalo Community	1.15			regularities			ertical (typ							
	-		+		-		an (type)							
- 'D' 'B' 'A'	'F'			xterior Falli	ng		nbraced C	himneys					eavy Vene	er
'E' "'H'	tidea		Ha	azards:			arapets			🗌 Арр	endages	i		
			6	COMMENTS			uner.							
		-Transfer		Jonniente										
'G'	-1													
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	M.M.	AL.												
A CON PERSONNEL	A	A.												
SKETCH			E	Additional	sketche	es or con	nments or	n separat	e page					
BA	SIC SCC	RE, MOD	FIERS, A		AL LE	EVEL	I SCOF	RE, S _L	1					
FEMA BUILDING TYPE Do Not W1 Know	W1A W2		S2 S3 BR) (LM)		S5 (URM	C1 (MRF)	C2 (SW)	C3 (URM	PC1 (TU)	PC2	RM1 (FD)	RM2 (RD)	URM	МН
				ŚW)	ÌNF)	. ,		INF)	()	45		. ,	4.0	
Basic Score 4.1 Severe Vertical Irregularity, VL1 -1.3	3.7 3.2 -1.3 -1.3		2.2 2.9 -1.0 -1.2		2.0 -0.9	1.7 -1.0	2.1 -1.1	1.4 -0.8	1.8 -1.0	1.5 -0.9	1.8 -1.0	1.8 -1.0	1.2 -0.8	2.2 NA
Moderate Vertical Irregularity, VL1 -0.8	-0.8 -0.8		0.6 -0.8		-0.6	-0.6	-0.6	-0.5	-0.6	-0.6	-0.6	-0.6	-0.5	NA
Plan Irregularity, P_{L1} -1.3	-1.2 -1.1		0.8 -1.0		-0.7	-0.7	-0.9	-0.6	-0.8	-0.7	-0.7	-0.7	-0.5	NA
Pre-Code -0.8 Post-Benchmark 1.5	-0.9 -0.9 1.9 2.3		·0.5 -0.7 1.4 1.0		-0.2 NA	-0.4 1.9	-0.7 2.1	-0.1 NA	-0.4 2.1	-0.3 2.4	-0.5 2.1	-0.5 2.1	-0.1 NA	-0.3 1.2
Soil Type A or B	0.6 0.9		0.9 0.3		0.9	0.6	0.8	0.7	0.9	0.7	0.8	0.8	0.6	0.9
Soil Type E (1-3 stories) 0.0	-0.1 -0.3	3 -0.4	0.5 0.0	-0.4	-0.5	-0.2	-0.2	-0.4	-0.5	-0.3	-0.4	-0.4	-0.3	-0.5
Soil Type E (> 3 stories) -0.5	-0.8 -1.2		0.7 NA		-0.6	-0.6	-0.8	-0.4	NA	-0.5	-0.6	-0.7	-0.3	NA
Minimum Score, Smin 1.6	1.2 0.8	0.5	0.5 0.9	0.5	0.5	0.3	0.3	0.3	0.3	0.2	0.3	0.3	0.2	1.4
FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:														
EXTENT OF REVIEW		OTHER I	HAZARD)S		ACT	ION RE	QUIR	ED					
Exterior: Partial All Sides		Are There H				Detaile	ed Struct	ural Eval	luation	Require	ed?			
Interior:	_ Entered	Detailed Str					es, unknov			ig type o	r other bu	uilding		
Soil Type Source:		Cut-off, i		(unless S _{L2} >			es, score l es. other h							
Geologic Hazards Source:		vn)												
		- •		is from taller adjacent Detailed Nonstructural Evaluation Recommended? (cl										
Contact Person:		building				Detaile	ed Nonsti	uctural	Evalua	tion Rec	ommeno	ded? (ch	eck one)	
Contact Person:)?	building	c hazards or int damage/	r Soil Type F /deterioratior	:	🗌 Ye	es, nonstru	uctural ha	azards i	dentified	that sho	uld be ev	valuated	
	•? □ No	building	c hazards or	r Soil Type F /deterioratior	:	□ Ye □ No	es, nonstru	uctural ha	azards i zards e	dentified xist that	that sho	uld be ev	,	а
LEVEL 2 SCREENING PERFORMED		building	c hazards or int damage/	r Soil Type F /deterioratior	:	☐ Ye ☐ No de	es, nonstru o, nonstru	uctural ha ctural ha luation is	azards i zards e s not ne	dentified xist that cessary	that sho may requ	uld be ev	valuated	a
LEVEL 2 SCREENING PERFORMED Yes, Final Level 2 Score, SL2	□ No □ No verified, sci	building Geologi Significa the strue	c hazards or int damage/ ctural systen ote the follo	r Soil Type F /deterioratior m	to T = Esti	☐ Ye ☐ No de ☐ No mated o	es, nonstru o, nonstru tailed eva o, no nons r unreliat	uctural ha ctural ha luation is tructural	azards i zards e not ne hazard	dentified xist that cessary s identifi	that sho may requ ed [o Not Kr	uld be ev ire mitig DNK	valuated	

Desc_sch11C Level 1 MODERATELY HIGH Seismicity

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100	et.						1	_						Z	Zip:			
	10						Oth	er Identi	ifiers:									
The second s	10						Bui											
	230					-		: tude:					ongitu	do.				
	1. 中部		and the second		AT THE OWNER	100	Ss:						Longitu S₁:	ue				
		N.	-					eener(s)	:				-	ate/Time	e:			
THE REAL		1					No.	Stories:	Abov	e Grade):	Belov	w Grade	:	Yea	r Built:	C	EST
TARE		THON T	NO		TE	-		al Floor litions:							Code	Year:		
- CASE			at Fair					upancy		one L	Yes, Y Comme		Emer. S	onvioon		otorio	Shelte	or
		V.		X		and the second		upancy	Indu	istrial	Office		School		G	overnmei		U
		- AND						T	Utili	•	Wareho			tial, #Ur		NK		
	Section 10	-			<u>.</u>		Sol	Туре:	□A Hard Rock	□B Avg Rock	Den: So	se S	tiff S	oft P			ume Type	D.
2nd St	2nd St		200	d St			Geo	logic Ha				-		-	-	Surf. R	upt.: Yes/N	No/DNK
					-			acency:			ounding						' It Building	
	Tumalo Commur School (Elementa	nity ary		100				gularitie			ertical (ty		ity) _					
	- 'D'	'B' '/	A' 1 ' ' '				- Evt	erior Fal	ling		an (type) nbraced		10			ling or H	leavy Ven	oor
	6							ards:	iiiig	D P	arapets	Chinney	5		pendages	-	leavy ven	
	H	'C	7				со	MMENT	S:	0	ther:							
ä.			-	-														
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	Statement of the local division of the local	-		-														
					. 7													
Martin Sta	in Rive	aller I	-															
					- 70													
ALL DESCRIPTION OF THE OWNER OF T	SKI	ETCH	A					Additiona	al sketch	es or cor	nments (n senar:	ate narie					
	0.11		ASIC	sco	RE, MOI													
FEMA BUILDING TYPE	Do Not	W1	W1A	W2	S1 (MRF)	S2 (BR)	S3 (LM)	S4 (RC	S5 (URM	C1 (MRF)	C2 (SW)	C3 (URM	PC1 (TU)	PC2	RM1 (FD)	RM2 (RD)	URM	МН
	Know						, ,	ŚW)	ÌNF)	. ,	. ,	ÌNF)	. ,		. ,	` '		
Basic Score Severe Vertical Irregularity, VL1		4.1 -1.3	3.7 -1.3	3.2 -1.3	2.3 -1.1	2.2 -1.0	2.9 -1.2	2.2 -1.0	2.0 -0.9	1.7 -1.0	2.1 -1.1	1.4 -0.8	1.8 -1.0	1.5 -0.9	1.8 -1.0	1.8 -1.0	1.2 -0.8	2.2 NA
Moderate Vertical Irregularity, V_{L1}		-0.8	-0.8	-0.8		-0.6	-0.8	-0.6	-0.6	-0.6	-0.6	-0.5	-0.6	-0.6	-0.6	-0.6	-0.5	NA
Plan Irregularity, PL1		-1.3	-1.2	-1.1		-0.8	-1.0	-0.8	-0.7	-0.7	-0.9	-0.6	-0.8	-0.7	-0.7	-0.7	-0.5	NA
Pre-Code Post-Benchmark		-0.8 1.5	-0.9 1.9	-0.9 2.3	-0.5 1.4	-0.5 1.4	-0.7 1.0	-0.6 1.9	-0.2 NA	-0.4 1.9	-0.7 2.1	-0.1 NA	-0.4 2.1	-0.3 2.4	-0.5 2.1	-0.5 2.1	<u>-0.1</u> NA	-0.3 1.2
Soil Type A or B		0.3	0.6	0.9	0.6	0.9	0.3	0.9	0.9	0.6	0.8	0.7	0.9	0.7	0.8	0.8	0.6	0.9
Soil Type E (1-3 stories)		0.0	-0.1	-0.3	-0.4	-0.5	0.0	-0.4	-0.5	-0.2	-0.2	-0.4	-0.5	-0.3	-0.4	-0.4	-0.3	-0.5
Soil Type E (> 3 stories)		-0.5	-0.8 1.2	-1.2		-0.7	NA	-0.7	-0.6	-0.6 0.3	-0.8	-0.4	NA	-0.5 0.2	-0.6	-0.7	-0.3 0.2	NA
Minimum Score, SMIN FINAL LEVEL 1 SCORE, S	1 1 > SMIN:	1.6	1.2	0.8	0.5	0.5	0.9	0.5	0.5	0.3	0.3	0.3	0.3	0.2	0.3	0.3	0.2	1.4
EXTENT OF REVIEW	_, _ •/////				OTHER	HA7		;		ACT	ION R	EQUIF	RED					
Exterior:	ial 🗆 i	All Sides	□ Aer	ial	Are There				4		ed Struc			Require	ed?			
Interior: 🗌 Non	ie 🗖 '	Visible			Detailed S									•	r other bi	uildina		
Drawings Reviewed: Yes Soil Type Source:		No				•••	· ·	nless SL2	>	🗌 Ye	es, score	less tha	n cut-off	3.7700				
Geologic Hazards Source:				-+		f, if kno 1 hazar		aller adja	cent		es, other o	hazards	present					
Contact Person:					buildir	ig						tructura	l Evalua	tion Rec	ommen	ded? (ch	neck one)	
LEVEL 2 SCREENING								Soil Type							I that sho			
Yes, Final Level 2 Score, S		JRIVIE					system	eterioratio	1110		o, nonstr	uctural h	azards e	xist that			ation, but	а
	Yes										etailed ev o, no nor				ed F			
Where info		cannot k			oonor shall	noto t	ha fallau	vina, EQ	T- Eati		,					_		
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					noie	le lunum	////U F		maienn	l numena	Die Dala			0 1401 61	10W		
	Moment-res raced frame	sisting fram	ne	RC = Re	einforced con hear wall			URM INF : TU = Tilt u	= Unreinfo			MH	= Manufa = Light me	ctured Ho	using F	D = Flexib	ole diaphrag diaphragm	

Desc_sch11D Level 1 MODERATELY HIGH Seismicity

					-		Add	ress:										
and the state of the	14							_							Zip:			
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	1220	annes ?			A		Bull	aing Na	me:									
	TH. NIME				_		Use						000	da:				
						IN PROD	Latin Ss:	ude:				!	Longitu	de:				
	Steller R. C				T. Balling			ener(s)				`	51:	ate/Tim	e:			
	A REAL POINT OF THE PARTY OF			-												r Built:		EST
		10		1			NO. Tota	Stories:	ADOV Area (so	e Grade		Beiov	N Grade		Code	Year:	L	1 EST
States and		in the	Trail of Los			and the second		itions:			Yes, Y	(ear(s) B	uilt:		- 0000	rear.		
Charles and the					A State		Occ	upancy		embly	Comme		Emer. S	ervices	D Hi	storic	Shelte	er
					E stale age	and the			Indu	strial	Office		School			overnmei	nt	
	The second	- Constants			THE REAL				Utilit	,	Wareho			itial, #Ur				
	1		Part -			and the second	Soil	Туре:	□A Hard	□B Avg	Den					NK DNK ass	ume Type I	n
	· · · · ·	a sugge		Calls .		-			Rock	Rock	Soi	1			Soil	DIVIN, 033	unie rype i	υ.
2nd/St	2nd St		2	2nd St			Geo	logic Ha	azards:	Liquefac	ction: Yes	s/No/DNł	< Lands	lide: Yes	/No/DNK	Surf. R	upt.: Yes/N	lo/DNK
	ALC: NO						Adja	cency:		D Po	ounding		Falling H	azards fr	om Taller	Adjacen	t Building	
E	Tumalo Comm School (Eleme	nunity ntary	1993					Jularitie	s:	U Ve	ertical (ty	pe/sever	ity)					
	- 'D'			- 33	2 3		- `				an (type)							
		'B'	'A'	'F' 👔	-			rior Fal	ling		nbraced	Chimney	is				eavy Vene	eer
'E'	weee 'Н'		H				Haza	ards:			arapets			🗌 App	pendages	6		
	2 -		C'	and the		2	<u> </u>	MMENT	¢.	0	ther:							
i .					-				3.									
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ALLA AL	AN AL	alle.	A	it.														
	Acres 1		Per		1													
	CIVI	ETCH			19.4			Additiona	al sketch	es or cor	nments c	on separa	ate page					
	201																	
	311		ASIC	scol	RE. MO	DIFIE	RS. AN				1 SCO	RE. S	11					
FEMA BUILDING TYPE	Do Not		ASIC W1A	SCOI	RE, MO	DIFIEI S2	RS, AI ^{S3}				1 SCO C2	RE, S	L1 PC1	PC2	RM1	RM2	URM	МН
FEMA BUILDING TYPE		B						ND FIN	IAL LE	EVEL '				PC2	RM1 (FD)	RM2 (RD)	URM	МН
Basic Score	Do Not	B/ W1 4.1	W1A 3.7	W2 3.2	S1 (MRF) 2.3	S2 (BR) 2.2	\$3 (LM) 2.9	ND FIN S4 (RC SW) 2.2	IAL LE S5 (URM INF) 2.0	C1 (MRF) 1.7	C2 (SW) 2.1	C3 (URM INF) 1.4	PC1 (TU) 1.8	1.5	(FD) 1.8	(RD) 1.8	1.2	2.2
Basic Score Severe Vertical Irregularity, VL1	Do Not Know	B/ W1 4.1 -1.3	W1A 3.7 -1.3	W2 3.2 -1.3	S1 (MRF) 2.3 -1.1	S2 (BR) 2.2 -1.0	S3 (LM) 2.9 -1.2	ND FIN S4 (RC SW) 2.2 -1.0	IAL LE S5 (URM INF) 2.0 -0.9	C1 (MRF) 1.7 -1.0	C2 (SW) 2.1 -1.1	C3 (URM INF) 1.4 -0.8	PC1 (TU) 1.8 -1.0	1.5 -0.9	(FD) 1.8 -1.0	(RD) 1.8 -1.0	1.2 -0.8	2.2 NA
Basic Score Severe Vertical Irregularity, V _{L1} Moderate Vertical Irregularity, V _L	Do Not Know	B/ W1 4.1 -1.3 -0.8	W1A 3.7 -1.3 -0.8	W2 3.2 -1.3 -0.8	S1 (MRF) 2.3 -1.1 -0.7	S2 (BR) 2.2 -1.0 -0.6	S3 (LM) 2.9 -1.2 -0.8	ND FIN (RC SW) 2.2 -1.0 -0.6	IAL LE (URM INF) 2.0 -0.9 -0.6	C1 (MRF) 1.7 -1.0 -0.6	C2 (SW) 2.1 -1.1 -0.6	C3 (URM INF) 1.4 -0.8 -0.5	PC1 (TU) 1.8 -1.0 -0.6	1.5 -0.9 -0.6	(FD) 1.8 -1.0 -0.6	(RD) 1.8 -1.0 -0.6	1.2 -0.8 -0.5	2.2 NA NA
Basic Score Severe Vertical Irregularity, V _{L1} Moderate Vertical Irregularity, V _L Plan Irregularity, P _{L1}	Do Not Know	B / W1 4.1 -1.3 -0.8 -1.3	W1A 3.7 -1.3 -0.8 -1.2	W2 3.2 -1.3 -0.8 -1.1	S1 (MRF) 2.3 -1.1 -0.7 -0.9	S2 (BR) 2.2 -1.0 -0.6 -0.8	S3 (LM) -1.2 -0.8 -1.0	ND FIN (RC SW) 2.2 -1.0 -0.6 -0.8	IAL LE (URM INF) 2.0 -0.9 -0.6 -0.7	C1 (MRF) 1.7 -1.0 -0.6 -0.7	C2 (SW) 2.1 -1.1 -0.6 -0.9	C3 (URM INF) 1.4 -0.8 -0.5 -0.6	PC1 (TU) 1.8 -1.0 -0.6 -0.8	1.5 -0.9 -0.6 -0.7	(FD) 1.8 -1.0 -0.6 -0.7	(RD) 1.8 -1.0 -0.6 -0.7	1.2 -0.8 -0.5 -0.5	2.2 NA NA NA
Basic Score Severe Vertical Irregularity, V _{L1} Moderate Vertical Irregularity, V _L	Do Not Know	B/ W1 4.1 -1.3 -0.8	W1A 3.7 -1.3 -0.8	W2 3.2 -1.3 -0.8	S1 (MRF) 2.3 -1.1 -0.7	S2 (BR) 2.2 -1.0 -0.6	S3 (LM) 2.9 -1.2 -0.8	ND FIN (RC SW) 2.2 -1.0 -0.6	IAL LE (URM INF) 2.0 -0.9 -0.6	C1 (MRF) 1.7 -1.0 -0.6	C2 (SW) 2.1 -1.1 -0.6	C3 (URM INF) 1.4 -0.8 -0.5	PC1 (TU) 1.8 -1.0 -0.6	1.5 -0.9 -0.6	(FD) 1.8 -1.0 -0.6	(RD) 1.8 -1.0 -0.6	1.2 -0.8 -0.5	2.2 NA NA
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_L Plan Irregularity, P_{L1} Pre-Code	Do Not Know	B /W1 4.1 -1.3 -0.8 -1.3 -0.8	W1A 3.7 -1.3 -0.8 -1.2 -0.9	W2 -1.3 -0.8 -1.1 -0.9	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5	S2 (BR) -1.0 -0.6 -0.8 -0.5	S3 (LM) -1.2 -0.8 -1.0 -0.7	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6	IAL LE (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4	1.5 -0.9 -0.6 -0.7 -0.3	(FD) 1.8 -1.0 -0.6 -0.7 -0.5	(RD) 1.8 -1.0 -0.6 -0.7 -0.5	1.2 -0.8 -0.5 -0.5 -0.1	2.2 NA NA NA -0.3
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories)	Do Not Know	B /W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1	W2 -1.3 -0.8 -1.1 -0.9 2.3	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0	ND FIN (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3	2.2 NA NA -0.3 1.2
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)	Do Not Know	B / 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8	W2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN	Do Not Know	B / W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1	W2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5
Basic Score Severe Vertical Irregularity, V_{L1} Moderate Vertical Irregularity, V_{L} Plan Irregularity, P_{L1} Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)	Do Not Know	B / 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8	W2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN	$\frac{\text{Do Not}}{\text{Know}}$	B / 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8	W2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9	S4 (RC SWW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	Do Not Know	B / W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 ial	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are Them	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ	S3 (LM) -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1	ND FIN S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Not	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are Them Detailed	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ Structur	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1 al Evalu	S4 (RC (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Evo	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 ial	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 0.5 TA 0.6 -0.7 0.5	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 CHAZ CHAZ CHAZ CHAZ CHAZ	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1 al Evalu ential (un	S4 (RC (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Evo pwn FEM less tha	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed?	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Noi Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source:	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 ial	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 0.5 1.4 0.6 -0.4 0.5 Detailed □	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ R HAZ C HAZ C HAZ	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS is That T al Evalu ential (un vn)	S4 (RC SW) 2.2 -1.0 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Evo pwn FEM less tha	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed?	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Noi Drawings Reviewed: Yes Soil Type Source: Par	Do Not Know	B / W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 ial	S1 (MRF) 2.3 -1.1 -0.7 -0.5 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ R HAZ Gamma R HAZ Gamma R HAZ	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1 al Evalu ential (un vn) Is from ta	S4 (RC SWW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Evo pwn FEM less tha hazards	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other bu	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3	2.2 NA NA -0.3 1.2 0.9 -0.5 NA
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Noi Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source:	$\begin{array}{c} \text{Do Not} \\ \text{Know} \end{array}$	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible No	W1A 3.7 -1.3 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 ial	S1 (MRF) 2.3 -1.1 -0.7 -0.9 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 C A A A A A A A A B A A B A A A A A A A A A A	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS ARDS ARDS Is That T al Evalu ential (un vn) Is from ta ards or S	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5 -0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 0.6 0.3 0 Etailo Detailo Detailo Detailo Detailo Detailo Detailo	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonst	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Evo pwn FEM less that hazards tructural f	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other but commend that sho	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	2.2 NA NA -0.3 1.2 0.9 -0.5 NA 1.4
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: □ Par Interior: □ Not Drawings Reviewed: □ Yes Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible No	W1A 3.7 -1.3 -0.9 1.9 0.6 -0.1 -0.8 1.2	W2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8 0.9 -0.3 -1.2 0.8	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ R HAZ Gamma R HAZ Gamma R HAZ	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1 al Evalu ential (un vn) Is from ta ards or S mage/de	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5 -0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 0.6 0.3 0.6 0.3 0.6 0.3	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonstru	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Evo pwn FEM less that hazards tructural huger	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards e	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Required tion Reco identified xist that	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other but commend that sho	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	2.2 NA NA -0.3 1.2 0.9 -0.5 NA 1.4
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: □ Par Interior: □ Not Drawings Reviewed: □ Yes Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENINC □ Yes, Final Level 2 Score, S 1	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible No	W1A 3.7 -1.3 -0.8 -1.2 -0.9 1.9 0.6 -0.1 -0.8 1.2 □ Aer □ Entu	w2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) -1.0 -0.6 -0.8 -0.5 -0.7 -0.5 -0.7 -0.5 C R HAZ ding pote ff, if knov g hazard ng ogic haza ficant dat	S3 (LM) 2.9 -1.2 -0.8 -1.0 -0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That 1 al Evalu ential (un vn) Is from ta ards or S mage/de	S4 (RC SW) 2.2 -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5 -0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 Detaild Ye Dye Octaild Ye No Detaild Ye No de	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonst	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 -0.4 -0.4 -0.4 -0.4 -0.4	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards i azards e is not ne	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.3 0.7 -0.5 0.2 Require ng type o tion Reco tidentified xist that cessary	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other but commented that sho may requ	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	2.2 NA NA -0.3 1.2 0.9 -0.5 NA 1.4
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: □ Par Interior: □ Noi Drawings Reviewed: □ Yes Soil Type Source: Geologic Hazards Source: Contact Person:	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible No	W1A 3.7 -1.3 -0.8 -1.2 -0.9 0.6 -0.1 -0.8 1.2	w2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) 2.2 -1.0 -0.6 -0.8 -0.5 1.4 0.9 -0.5 -0.7 0.5 C R HAZ. R HAZ. R HAZ. R HAZ. R HAZ. R HAZ.	S3 (LM) 2.9 -1.2 -0.8 -1.0 0.7 1.0 0.3 0.0 NA 0.9 ARDS ARDS ARDS Is That T al Evalu ential (un vn) Is from ta ards or S mage/de system	S4 (RC SW) 2.2 -1.0 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile □ Yee □ Net □ Yee □ Net □ Yee □ Net □ Net	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o de Struc es, nonstru- tailed ev o, no non	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Evo bwn FEM less that hazards tructural h aluation istructura	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards e is not ne al hazard	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.7 -0.3 -0.5 0.2 Require tion Rec tion Rec tidentified xist that cessary ti dentified	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other but commend that sho may requ ed	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding ded? (ch uild be e' uire mitig	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	2.2 NA NA -0.3 1.2 0.9 -0.5 NA 1.4
Basic Score Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1 Plan Irregularity, PL1 Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: □ Par Interior: □ Noi Drawings Reviewed: □ Yes Soil Type Source: Geologic Hazards Source: Contact Person:	Do Not Know	B/ W1 4.1 -1.3 -0.8 -1.3 -0.8 1.5 0.3 0.0 -0.5 1.6 All Sides Visible No	W1A 3.7 -1.3 -0.8 -1.2 -0.9 0.6 -0.1 -0.8 1.2 O Aer Ento	w2 3.2 -1.3 -0.8 -1.1 -0.9 2.3 0.9 -0.3 -1.2 0.8	S1 (MRF) 2.3 -1.1 -0.7 -0.9 -0.5 1.4 0.6 -0.4 -0.7 0.5	S2 (BR) -1.0 -0.6 -0.8 -0.5 -0.7 -0.5 -0.7 -0.5 R HAZ R HAZ R HAZ R HAZ R HAZ I note th	S3 (LM) 2.9 -1.2 -0.8 -1.0 0.7 1.0 0.3 0.0 NA 0.9 ARDS Is That I al Evalu ential (un vn) Is from ta ards or S mage/de system e follow	State State (RC (RC SWW) -1.0 -0.6 -0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	IAL LE S5 (URM INF) 2.0 -0.9 -0.6 -0.7 -0.2 NA 0.9 -0.5 -0.6 0.5	C1 (MRF) 1.7 -1.0 -0.6 -0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile □ Yee □ Net □ Yee □ Net □ Yee □ Net □ Net	C2 (SW) 2.1 -1.1 -0.6 -0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 CON R ed Struc es, unkno es, score es, other o ed Nons es, nonstru- tailed ev o, no non r unrelia	C3 (URM INF) 1.4 -0.8 -0.5 -0.6 -0.1 NA 0.7 -0.4 -0.4 -0.4 0.3 EQUIF tural Ev. bwn FEM less tha hazards tructural h aluation istructura mH	PC1 (TU) 1.8 -1.0 -0.6 -0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation I Evalua hazards e is not ne al hazards 0 RED	1.5 -0.9 -0.6 -0.7 -0.3 2.4 0.7 -0.3 -0.5 0.7 -0.3 -0.5 0.2 Required tion Rec tion Rec tidentified xist that ccessary tis identified DNK = D Ctured Ho	(FD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other but commend t that sho may requ ed [to Not Kr using Files]	(RD) 1.8 -1.0 -0.6 -0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding ded? (ch uild be e' uire mitig DNK Tow D = Flexib	1.2 -0.8 -0.5 -0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	2.2 NA NA -0.3 1.2 0.9 -0.5 NA 1.4

Desc_sch11E Level 1 MODERATELY HIGH Seismicity

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FEMA BUILDING TYPE	Do Not		W1A	W2	RE, MC	S2	5, AI	S4			C2		L1 PC1	PC2	RM1	RM2	URM	МН
TEMA BUILDING TIPE	Know				(MRF)		(LM)	(RC SW)	(URM INF)	(MRF)	(SW)	(URM INF)	(TU)	102	(FD)	(RD)		
Basic Score		4.1	3.7	3.2	2.3	2.2	2.9	2.2	2.0	1.7	2.1	1.4	1.8	1.5	1.8	1.8	1.2	2.2
Severe Vertical Irregularity, VL1		-1.3	-1.3	-1.3	-1.1	-1.0	-1.2	-1.0	-0.9	-1.0	-1.1	-0.8	-1.0	-0.9	-1.0	-1.0	-0.8	NA
Moderate Vertical Irregularity, V _{L1} Plan Irregularity, P _{L1}		-0.8 -1.3	-0.8 -1.2	-0.8 -1.1		-0.6 -0.8	-0.8 -1.0	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.9	-0.5 -0.6	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.7	-0.5 -0.5	NA NA
Pre-Code		-0.8	-0.9	-0.9	-0.9	-0.0	-0.7	-0.6	-0.7	-0.7	-0.9	-0.0	-0.8	-0.7	-0.7	-0.7	-0.5	-0.3
Post-Benchmark		1.5	1.9	2.3	1.4	1.4	1.0	1.9	NA	1.9	2.1	NA	2.1	2.4	2.1	2.1	NA	1.2
Soil Type A or B		0.3	0.6	0.9	0.6	0.9	0.3	0.9	0.9	0.6	0.8	0.7	0.9	0.7	0.8	0.8	0.6	0.9
Soil Type E (1-3 stories)		0.0	-0.1	-0.3		-0.5	0.0	-0.4	-0.5	-0.2	-0.2	-0.4	-0.5	-0.3	-0.4	-0.4	-0.3	-0.5
Soil Type E (> 3 stories)		-0.5 1.6	-0.8 1.2	-1.2 0.8	-0.7	-0.7 0.5	NA 0.9	-0.7 0.5	-0.6 0.5	-0.6 0.3	-0.8 0.3	-0.4 0.3	NA 0.3	-0.5 0.2	-0.6 0.3	-0.7 0.3	-0.3 0.2	NA 1.4
Minimum Score, S _{MIN}			1.2	0.0	0.0	0.0	0.9	0.5	0.5	0.3	0.3	0.3	0.3	0.2	0.3	0.3	0.2	1.4
FINAL LEVEL 1 SCORE, S	.1 ≥ 3 min:									1								
EXTENT OF REVIEW					OTHE		-				ION R	-						
Exterior: Part	-	All Sides	_		Are The				1				aluation	•				
Interior: Interior: Non Drawings Reviewed: Yes			Ent	ered	Detailed										r other bu	uilding		
Soil Type Source:						nding pot off, if knov	ential (un wn)	IIESS SL2	>				n cut-off present					
Geologic Hazards Source:							ds from ta	aller adja	cent			1020105	PICOCIIL					
Contact Person:					build	ling					ed Nons	tructura	l Evalua	tion Rec	ommend	ded? (cł	neck one)	
LEVEL 2 SCREENING	PFRF	ORMF	D?				ards or S mage/de			□ Ye	es, nonst	ructural	hazards i	identified	l that sho	uld be e	valuated	
Yes, Final Level 2 Score, S			П N	ю		structural									may requ	uire mitig	ation, but	а
	Yes			-									is not ne al hazard	,	ed Г			
Where info		cannot b		-	ener sha	ll note tl	ne follow	ing: ES	T = Esti	_	,							
Legend: MRF = 1	Noment-res	sisting fram	ne	RC = Re	einforced co		ι	JRM INF =	= Unreinfo	rced maso		MH	= Manufa	ctured Ho	using Fl	D = Flexik	ole diaphrag	
BR = Br	aced frame			SW = SI	hear wall		1	ΓU = Tilt u	р			LM	= Light me	etal	R	D = Rigid	diaphragm	ı

Desc_sch11F Level 1 MODERATELY HIGH Seismicity

	Address:	
		Zip:
	Other Identifiers:	
	Building Name:	
	Use:	Longitudo:
	Ss:	Longitude: S1:
	Screener(s):	
	No. Storios: Ab	
	Total Floor Area (
		None Ves, Year(s) Built:
		ssembly Commercial Emer. Services Historic Shelter
		dustrial Office School 🗋 Government
		ility Warehouse Residential, # Units:
	Soil Type: A	B C D E F DNK Avg Dense Stiff Soft Poor If DNK, assume Type D.
	Rock	
2nd St 2nd St 2nd St	Geologic Hazards	: Liquefaction: Yes/No/DNK Landslide: Yes/No/DNK Surf. Rupt.: Yes/No/DNK
	Adjacency:	Pounding Falling Hazards from Taller Adjacent Building
Tunalo Community School (Elementary	Irregularities:	☐ Vertical (type/severity)
		Plan (type)
- 'D' 'B' 'A' 'F'	Exterior Falling	Unbraced Chimneys Heavy Cladding or Heavy Veneer
	Hazards:	Parapets Appendages Other:
	COMMENTS:	
'G'		
this is and the ask the		
SKETCH	Additional sket	thes or comments on separate page
BASIC SCORE, MODIF		
FEMA BUILDING TYPE Do Not W1 W1A W2 S1 S2	S3 S4 S5	C1 C2 C3 PC1 PC2 RM1 RM2 URM MH
Know (MRF) (BR	(LM) (RC (URM SW) INF)	(MRF) (SW) (URM (TU) (FD) (RD)
Basic Score 4.1 3.7 3.2 2.3 2.2	2.9 2.2 2.0	1.7 2.1 1.4 1.8 1.5 1.8 1.8 1.2 2.2
Severe Vertical Irregularity, V_{L1} -1.3 -1.3 -1.3 -1.1 -1.1 Moderate Vertical Irregularity, V_{L1} -0.8 -0.8 -0.8 -0.7 -0.1		-1.0 -1.1 -0.8 -1.0 -0.9 -1.0 -1.0 -0.8 NA -0.6 -0.6 -0.5 -0.6 -0.6 -0.6 -0.6 -0.6 -0.5 NA
Plan Irregularity, P_{L1} -1.3 -1.2 -1.1 -0.9 -0.4		-0.7 -0.9 -0.6 -0.8 -0.7 -0.7 -0.7 -0.5 NA
Pre-Code -0.8 -0.9 -0.9 -0.5 -0.4	-0.7 -0.6 -0.2	-0.4 -0.7 -0.1 -0.4 -0.3 -0.5 -0.5 -0.1 -0.3
Post-Benchmark 1.5 1.9 2.3 1.4 1.4	1.0 1.9 NA	1.9 2.1 NA 2.1 2.4 2.1 2.1 NA 1.2
Soil Type A or B 0.3 0.6 0.9 0.6 0.5 Soil Type E (1-3 stories) 0.0 -0.1 -0.3 -0.4 -0.4	0.3 0.9 0.9 0.9 0.0 -0.4 -0.5	0.6 0.8 0.7 0.9 0.7 0.8 0.8 0.6 0.9 -0.2 -0.2 -0.4 -0.5 -0.3 -0.4 -0.4 -0.3 -0.5
Soil Type E (>3 stories) -0.5 -0.8 -1.2 -0.7 -0.7		-0.2 -0.2 -0.4 -0.5 -0.6 -0.7 -0.3 -0.5
Minimum Score, Smin 1.6 1.2 0.8 0.5 0.5	0.9 0.5 0.5	0.3 0.3 0.3 0.3 0.2 0.3 0.3 0.2 1.4
FINAL LEVEL 1 SCORE, $S_{L1} \ge S_{MIN}$:		
EXTENT OF REVIEW OTHER HA	ZARDS	ACTION REQUIRED
	ards That Trigger A	Detailed Structural Evaluation Required?
Interior: Interi	ural Evaluation?	Yes, unknown FEMA building type or other building
	otential (unless S_{L2} >	Yes, score less than cut-off
	own) ards from taller adjacent	 Yes, other hazards present No
Contact Person: building		Detailed Nonstructural Evaluation Recommended? (check one)
	azards or Soil Type F damage/deterioration to	Yes, nonstructural hazards identified that should be evaluated
		No, nonstructural hazards exist that may require mitigation, but a detailed evolution is not reserved.
Nonstructural hazards?	-	detailed evaluation is not necessary No, no nonstructural hazards identified
Where information cannot be verified, screener shall note	the following: $FST = Fc$	
Legend: MRF = Moment-resisting frame RC = Reinforced concrete	-	forced masonry infill MH = Manufactured Housing FD = Flexible diaphragm

Desc_sch11G Level 1 MODERATELY HIGH Seismicity

	H Sale			1		1	Add	ress:										
	Contraction of the second seco	E W	1					_						Z	'ip:			
THE ELECTRON	The states	2	//		1	DE	Othe	er Identi	fiers:									
NY44	At .	//		10	Contra		Build	ding Na	me:									
	5	-	-		-		Use:											
	1						Latit	tude:				I	ongitu	ae:				
						11000	Ss:	()				:	S₁:					
				間		1 44	Scre	ener(s)					Da	ate/ l ime	:			
Carton - Bar	-				-		No.	Stories:	Abov	e Grade	:	Belov	v Grade	:	Year	Built:		EST
							Tota	II FIOOR	Area (so	ι.π.):	Yes, Y				_ Code	Year:		
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and the second second					Provenue.		000	upancy.		strial	Office		School			overnmen		51
		12		-	See.				Utilit	у	Wareho	use	Residen	itial, #Ur	nits:			
		0				Real Property in	Soil	Туре:	□A	□в						NK		
ů.	the second	Ser.	S. Series	and the second	•		-		Hard Rock	Avg Rock	Dens Soi				oor <i>If I</i> Soil	DNK, assi	ите Туре	D.
2nd St	2nd St		2nd St	. 11			Geo	logic Ha			ction; Yes	-		-	-	Surf Ri	int · Yes/I	Jo/DNK
	i Car		States -		-			icency:	120103.		ounding				om Taller		•	
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	hool (Elementary	- 10-	-		and a state		Irreg	gularitie	s:		an (type)		ity)					
	'D' 'I	B' 'A'	·		- An		Exto	rior Fal	lina		nbraced				avy Clado	lina or H	aavv Van	۵۵r
				- HINE	-			ards:	iiiig		arapets	Chinney	3		pendages		eavy ven	661
'E' '+	ł'	'C'	1												0			
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	'G'	V	-		-													
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	SKE	ТСН	A BANK					Additiona	al sketche	es or cor	nments o	n separa	ate page					
		В	ASIC	SCOF	RE, MO	DIFIEF	RS, AN		IAL LE		1 SCO	RE, S	L1					
FEMA BUILDING TYPE	Do Not	W1	W1A	W2	S1 (MRF)	S2	S3	S4	S5	C1	C2	C3	PC1	PC2	RM1	RM2	URM	MH
	Know					(BR)	(LM)	(RC SW)	(URM INF)	(MRF)	(SW)	(URM INF)	(TU)		(FD)	(RD)		
Basic Score		4.1 -1.3	3.7	3.2	2.3	2.2	2.9 -1.2	2.2	2.0	1.7	2.1	1.4	1.8	1.5	1.8	1.8	1.2 -0.8	2.2
Severe Vertical Irregularity, VL1 Moderate Vertical Irregularity, VL1		-1.3 -0.8	-1.3 -0.8	-1.3 -0.8	-1.1 -0.7	-1.0 -0.6	-1.2 -0.8	-1.0 -0.6	-0.9 -0.6	-1.0 -0.6	-1.1 -0.6	-0.8 -0.5	-1.0 -0.6	-0.9 -0.6	-1.0	-1.0	-0.8	NA
Plan Irregularity, P_{L1}		-1.3	-1.2	-1.1	-0.9	-0.8										-0.6	-0.5	NΔ
		1.0			-0.5		-1.0	-0.8	-0.7	-0.7	-0.9	-0.6	-0.8	-0.7	-0.6 -0.7	-0.6 -0.7	-0.5 -0.5	NA NA
Pre-Code		-0.8	-0.9	-0.9	-0.5	-0.5	-1.0 -0.7		-0.7 -0.2									
Pre-Code Post-Benchmark		-0.8 1.5	-0.9 1.9	-0.9 2.3	-0.5 1.4	-0.5 1.4	-0.7 1.0	-0.8 -0.6 1.9	-0.2 NA	-0.7 -0.4 1.9	-0.9 -0.7 2.1	-0.6 -0.1 NA	-0.8 -0.4 2.1	-0.7 -0.3 2.4	-0.7 -0.5 2.1	-0.7 -0.5 2.1	-0.5 -0.1 NA	NA -0.3 1.2
Pre-Code Post-Benchmark Soil Type A or B		-0.8 1.5 0.3	-0.9 1.9 0.6	-0.9 2.3 0.9	-0.5 1.4 0.6	-0.5 1.4 0.9	-0.7 1.0 0.3	-0.8 -0.6 1.9 0.9	-0.2 NA 0.9	-0.7 -0.4 1.9 0.6	-0.9 -0.7 2.1 0.8	-0.6 -0.1 NA 0.7	-0.8 -0.4 2.1 0.9	-0.7 -0.3 2.4 0.7	-0.7 -0.5 2.1 0.8	-0.7 -0.5 2.1 0.8	-0.5 -0.1 NA 0.6	NA -0.3 1.2 0.9
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories)		-0.8 1.5 0.3 0.0	-0.9 1.9 0.6 -0.1	-0.9 2.3 0.9 -0.3	-0.5 1.4 0.6 -0.4	-0.5 1.4 0.9 -0.5	-0.7 1.0 0.3 0.0	-0.8 -0.6 1.9 0.9 -0.4	-0.2 NA 0.9 -0.5	-0.7 -0.4 1.9 0.6 -0.2	-0.9 -0.7 2.1 0.8 -0.2	-0.6 -0.1 NA 0.7 -0.4	-0.8 -0.4 2.1 0.9 -0.5	-0.7 -0.3 2.4 0.7 -0.3	-0.7 -0.5 2.1 0.8 -0.4	-0.7 -0.5 2.1 0.8 -0.4	-0.5 -0.1 NA 0.6 -0.3	NA -0.3 1.2 0.9 -0.5
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)		-0.8 1.5 0.3 0.0 -0.5	-0.9 1.9 0.6 -0.1 -0.8	-0.9 2.3 0.9 -0.3 -1.2	-0.5 1.4 0.6 -0.4 -0.7	-0.5 1.4 0.9 -0.5 -0.7	-0.7 1.0 0.3 0.0 NA	-0.8 -0.6 1.9 0.9 -0.4 -0.7	-0.2 NA 0.9 -0.5 -0.6	-0.7 -0.4 1.9 0.6 -0.2 -0.6	-0.9 -0.7 2.1 0.8 -0.2 -0.8	-0.6 -0.1 NA 0.7 -0.4 -0.4	-0.8 -0.4 2.1 0.9 -0.5 NA	-0.7 -0.3 2.4 0.7 -0.3 -0.5	-0.7 -0.5 2.1 0.8 -0.4 -0.6	-0.7 -0.5 2.1 0.8 -0.4 -0.7	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, <i>Smin</i>	Star 2 C.m.4	-0.8 1.5 0.3 0.0	-0.9 1.9 0.6 -0.1	-0.9 2.3 0.9 -0.3	-0.5 1.4 0.6 -0.4	-0.5 1.4 0.9 -0.5	-0.7 1.0 0.3 0.0	-0.8 -0.6 1.9 0.9 -0.4	-0.2 NA 0.9 -0.5	-0.7 -0.4 1.9 0.6 -0.2	-0.9 -0.7 2.1 0.8 -0.2	-0.6 -0.1 NA 0.7 -0.4	-0.8 -0.4 2.1 0.9 -0.5	-0.7 -0.3 2.4 0.7 -0.3	-0.7 -0.5 2.1 0.8 -0.4	-0.7 -0.5 2.1 0.8 -0.4	-0.5 -0.1 NA 0.6 -0.3	NA -0.3 1.2 0.9 -0.5
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, <i>Smin</i> FINAL LEVEL 1 SCORE, S	SL1≥ SMIN:	-0.8 1.5 0.3 0.0 -0.5	-0.9 1.9 0.6 -0.1 -0.8	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5	-0.5 1.4 0.9 -0.5 -0.7 0.5	-0.7 1.0 0.3 0.0 NA 0.9	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3	-0.7 -0.3 2.4 0.7 -0.3 -0.5	-0.7 -0.5 2.1 0.8 -0.4 -0.6	-0.7 -0.5 2.1 0.8 -0.4 -0.7	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, Smin FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW		-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5	-0.5 1.4 0.9 -0.5 -0.7 0.5	-0.7 1.0 0.3 0.0 NA 0.9	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	tial 🔲 A	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	tial 🗌 /	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ Hazard Structura	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source:	tial 🗌 A ne 🔲 V	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wwn FEM less that	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	tial 🗌 A ne 🔲 V	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S □ Poun cut-oi □ Fallin	-0.5 1.4 0.9 -0.5 -0.7 0.5 X HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un rn)	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wwn FEM less that	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source:	tial 🗌 A ne 🔲 V	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed 3 Poun cut-ol Fallin buildi	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ Hazard Structura ding pote f, if know	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un m) s from ta	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Detaile Detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wm FEM less that hazards	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off present	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2	NA -0.3 1.2 0.9 -0.5 NA
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior:	tial / ne \ s M	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun cut-oi Fallin buildi Geolo	-0.5 1.4 0.9 -0.5 -0.7 0.5 X HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ A HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un rn) s from ta	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Detaile Detaile Ye	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION RI ed Struct es, unkno es, score es, other o ed Nonst	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wm FEM less that hazards tructural h	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir present I Evalua nazards i	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require ng type o tion Rec	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other bu	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2 eck one) raluated	NA -0.3 1.2 0.9 -0.5 NA 1.4
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source: Contact Person:	tial / ne \ s t	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun cut-of Fallin buildi Geolo Signit	-0.5 1.4 0.9 -0.5 -0.7 0.5 X HAZ A HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un n) s from ta urds or Sin mage/dei	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye No Detaile Ye No	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION RI ed Struct es, unkno es, score es, other o ed Nonstruct o, nonstruct	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva less that hazards tructural hazards	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir present I Evalua nazards i azards e	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other bu	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2 eck one) raluated	NA -0.3 1.2 0.9 -0.5 NA 1.4
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Par Interior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score, S	tial / ne \ s t	-0.8 1.5 0.3 0.0 -0.5 1.6	-0.9 1.9 0.6 -0.1 -0.8 1.2	-0.9 2.3 0.9 -0.3 -1.2 0.8 ial ered	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun cut-of Fallin buildi Geolo Signit	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ R HAZ A HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un n) s from ta urds or Sin mage/dei	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Detaile Ye Detaile Ye No Detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION RI ed Struct es, unkno es, score es, other o ed Nonst	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva won FEM less that hazards tructural h actural hazards	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off present I Evalua nazards i azards e is not ne	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that increasary	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 r other bu	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2 eck one) raluated	NA -0.3 1.2 0.9 -0.5 NA 1.4
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score, S Nonstructural hazards?	tial / ne \ s S PERF(S _{L2}] Yes	-0.8 1.5 0.3 0.0 -0.5 1.6 All Sides /isible No	-0.9 1.9 0.6 -0.1 -0.8 1.2 Aer Entu	-0.9 2.3 0.9 -0.3 -1.2 0.8 ial ered	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun cut-ol Fallin buildi Geold Signii the st	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ A HAZ A HAZ A HAZ C HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un rn) s from ta indisor So mage/def system	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5	-0.2 NA 0.9 -0.5 -0.6 0.5 > cent F n to	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye No Detaile Ye No Cotaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION RI ed Struct es, unkno es, score es, other o ed Nonsi es, nonstru o, nonstru o, no non	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wm FEM less than hazards tructural h actural hazards	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off present I Evalua hazards i azards e is not ne al hazard	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that cessary Is identified	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 ed? r other bu	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding uid be ev irre mitiga	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2 eck one) raluated	NA -0.3 1.2 0.9 -0.5 NA 1.4
Pre-Code Post-Benchmark Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Par Interior: Nor Drawings Reviewed: Yes Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score, S Nonstructural hazards? Where inf Legend: MRF =	tial / he \ 5 6 PERFC 5 L2	-0.8 1.5 0.3 0.0 -0.5 1.6 All Sides /isible No	-0.9 1.9 0.6 -0.1 -0.8 1.2 Aer D? N e verifie e	-0.9 2.3 0.9 -0.3 -1.2 0.8 ial ered o o o o	-0.5 1.4 0.6 -0.4 -0.7 0.5 OTHEF Are There Detailed S Poun cut-of Fallin buildi Geolo Signit the st ener shal	-0.5 1.4 0.9 -0.5 -0.7 0.5 R HAZ R HAZ A HAZ	-0.7 1.0 0.3 0.0 NA 0.9 ARDS s That T al Evalue ential (un rn) s from ta urds or So mage/def system e follow	-0.8 -0.6 1.9 0.9 -0.4 -0.7 0.5 Frigger A ation? eless SL2 aller adja oil Type i terioratio	-0.2 NA 0.9 -0.5 -0.6 0.5 > cent F n to T = Esti - Unreinfo	-0.7 -0.4 1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Detaile Ye Detaile Ye No detaile	-0.9 -0.7 2.1 0.8 -0.2 -0.8 0.3 ION RI ed Struct es, unkno es, score es, other o red Nonst tailed eva o, no non r unrelia	-0.6 -0.1 NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva wn FEM less than hazards tructural hazards tructural hazards	-0.8 -0.4 2.1 0.9 -0.5 NA 0.3 RED aluation A buildir n cut-off present I Evalua nazards i azards e is not ne al hazard	-0.7 -0.3 2.4 0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that cessary Is identified DNK = D ctured Ho	-0.7 -0.5 2.1 0.8 -0.4 -0.6 0.3 r other bu commence that sho may requ ed [o Not Kr using Fi	-0.7 -0.5 2.1 0.8 -0.4 -0.7 0.3 uilding uild be ev uire mitiga DNK tow D = Flexibl	-0.5 -0.1 NA 0.6 -0.3 -0.3 0.2 eck one) raluated	NA -0.3 1.2 0.9 -0.5 NA 1.4 a

Desc_sch11H Level 1 MODERATELY HIGH Seismicity

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FEMA BUILDING TYPE	Do Not	W1	W1A	W2	S1	S2	S 3	S4	S5	C1	C2	C3	PC1	PC2	RM1	RM2	URM	МН
	Know				(MRF)	(BR)	(LM)	(RC SW)	(URM INF)	(MRF)	(SW)	(URM INF)	(TU)		(FD)	(RD)		
Basic Score		4.1	3.7	3.2	2.3	2.2	2.9	2.2	2.0	1.7	2.1	1.4	1.8	1.5	1.8	1.8	1.2	2.2
Severe Vertical Irregularity, VL1		-1.3	-1.3 -0.8	-1.3 -0.8	-1.1	-1.0	-1.2	-1.0	-0.9	-1.0 -0.6	-1.1	-0.8	-1.0	-0.9	-1.0	-1.0	-0.8	NA
Moderate Vertical Irregularity, V _L Plan Irregularity, P _{L1}	1	-0.8 -1.3	-0.0 -1.2	-0.0	-0.7 -0.9	-0.6 -0.8	-0.8 -1.0	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.9	-0.5 -0.6	-0.6 -0.8	-0.6 -0.7	-0.6 -0.7	-0.6 -0.7	-0.5 -0.5	NA NA
Pre-Code		-0.8	-0.9	-0.9	-0.5	-0.5	-0.7	-0.6	-0.2	-0.4	-0.7	-0.1	-0.4	-0.3	-0.5	-0.5	-0.1	-0.3
											0.1	-0.1	0.1					
Post-Benchmark		1.5	1.9	2.3	1.4	1.4	1.0	1.9	NA	1.9	2.1	NA	2.1	2.4	2.1	2.1	NA	1.2
Soil Type A or B		0.3	0.6	0.9	0.6	0.9	0.3	0.9	NA 0.9	1.9 0.6	2.1 0.8	NA 0.7	2.1 0.9	0.7	0.8	0.8	0.6	0.9
Soil Type A or B Soil Type E (1-3 stories)		0.3 0.0	0.6 -0.1	0.9 -0.3	0.6 -0.4	0.9 -0.5	0.3 0.0	0.9 -0.4	NA 0.9 -0.5	1.9 0.6 -0.2	2.1 0.8 -0.2	NA 0.7 -0.4	2.1 0.9 -0.5	0.7 -0.3	0.8 -0.4	0.8 -0.4	0.6 -0.3	0.9 -0.5
Soil Type A or B		0.3	0.6	0.9	0.6	0.9	0.3	0.9	NA 0.9	1.9 0.6	2.1 0.8	NA 0.7	2.1 0.9	0.7	0.8	0.8	0.6	0.9
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories)	SL1 ≥ Smin:	0.3 0.0 -0.5 1.6	0.6 -0.1 -0.8	0.9 -0.3 -1.2	0.6 -0.4 -0.7	0.9 -0.5 -0.7	0.3 0.0 NA	0.9 -0.4 -0.7	NA 0.9 -0.5 -0.6	1.9 0.6 -0.2 -0.6	2.1 0.8 -0.2 -0.8	NA 0.7 -0.4 -0.4	2.1 0.9 -0.5 NA	0.7 -0.3 -0.5	0.8 -0.4 -0.6	0.8 -0.4 -0.7	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, S _{MIN} FINAL LEVEL 1 SCORE, S		0.3 0.0 -0.5 1.6	0.6 -0.1 -0.8	0.9 -0.3 -1.2 0.8	0.6 -0.4 -0.7 0.5	0.9 -0.5 -0.7 0.5	0.3 0.0 NA 0.9	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6	1.9 0.6 -0.2 -0.6 0.3	2.1 0.8 -0.2 -0.8 0.3	NA 0.7 -0.4 -0.4 0.3	2.1 0.9 -0.5 NA 0.3	0.7 -0.3 -0.5	0.8 -0.4 -0.6	0.8 -0.4 -0.7	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, Smin FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW		0.3 0.0 -0.5 1.6	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8	0.6 -0.4 -0.7 0.5	0.9 -0.5 -0.7 0.5	0.3 0.0 NA 0.9	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3	2.1 0.8 -0.2 -0.8 0.3	NA 0.7 -0.4 -0.4 0.3	2.1 0.9 -0.5 NA 0.3	0.7 -0.3 -0.5 0.2	0.8 -0.4 -0.6 0.3	0.8 -0.4 -0.7	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW	rtial 🔲 .	0.3 0.0 -0.5 1.6	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8	0.6 -0.4 -0.7 0.5 OTHE	0.9 -0.5 -0.7 0.5	0.3 0.0 NA 0.9 ARDS s That 1	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc	NA 0.7 -0.4 -0.4 0.3 EQUIF	2.1 0.9 -0.5 NA 0.3	0.7 -0.3 -0.5 0.2	0.8 -0.4 -0.6 0.3	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: No Drawings Reviewed: Ye	rtial	0.3 0.0 -0.5 1.6	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8 ial ered	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed	0.9 -0.5 -0.7 0.5 R HAZA re Hazard Structura nding pote	0.3 0.0 NA 0.9 ARDS s That T al Evalu	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva own FEM less tha	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off	0.7 -0.3 -0.5 0.2 Require	0.8 -0.4 -0.6 0.3	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: No Drawings Reviewed: Ye Soil Type Source:	rtial	0.3 0.0 -0.5 1.6 All Sides Visible	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8 ial ered	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Pour cut-c	0.9 -0.5 -0.7 0.5 R HAZ/ re Hazard Structura nding pote off, if know	0.3 0.0 NA 0.9 ARDS s That 1 al Evalu ential (un m)	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Eva own FEM less tha	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off	0.7 -0.3 -0.5 0.2 Require	0.8 -0.4 -0.6 0.3	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: No Drawings Reviewed: Ye	rtial	0.3 0.0 -0.5 1.6 All Sides Visible	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8 ial ered	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Pour cut-c	0.9 -0.5 -0.7 0.5 R HAZA re Hazard Structura ading pote off, if know ng hazard	0.3 0.0 NA 0.9 ARDS s That 1 al Evalu ential (un m)	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Ye Ye	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o	NA 0.7 -0.4 <u>-0.4</u> 0.3 EQUIF tural Events tural Events tural Events tural Events tural Events	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present	0.7 -0.3 -0.5 0.2 Require	0.8 -0.4 -0.6 0.3	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3 0.2	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: No Drawings Reviewed: Ye Soil Type Source: Geologic Hazards Source: Contact Person:	rtial ne s	0.3 0.0 -0.5 1.6 All Sides Visible No	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Pour cut-c Detailed Fallin build	0.9 -0.5 -0.7 0.5 R HAZ re Hazard Structura nding pote ff, if know ng hazard ing ogic haza	0.3 0.0 NA 0.9 ARDS s That T al Evalu ential (un m) s from ta	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Ye No Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Evo wwn FEM less tha hazards	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present	0.7 -0.3 -0.5 0.2 Require	0.8 -0.4 -0.6 0.3 ed? r other bu	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3 0.2	0.9 -0.5 NA
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: No Drawings Reviewed: Ye Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING	rtial ne s G PERF(0.3 0.0 -0.5 1.6 All Sides Visible No	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Detailed Fallin build Geol Sign	0.9 -0.5 -0.7 0.5 R HAZA re Hazard Structura nding pote off, if know ng hazard ing ogic haza ificant dar	0.3 0.0 NA 0.9 ARDS s That 1 al Evalu ential (un m) s from ta urds or S mage/de	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye No Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonst	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Ev: own FEM less tha hazards tructural f	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards	0.7 -0.3 -0.5 0.2 Require ng type o tion Rec	0.8 -0.4 -0.6 0.3 ed? r other bu	0.8 -0.4 -0.7 0.3	0.6 -0.3 -0.3 0.2	0.9 -0.5 NA 1.4
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: Pa Interior: No Drawings Reviewed: Ye Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score,	rtial ne s G PERF(SL2	0.3 0.0 -0.5 1.6 All Sides Visible No	0.6 -0.1 -0.8 1.2	0.9 -0.3 -1.2 0.8 ial ered	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Detailed Fallin build Geol Sign	0.9 -0.5 -0.7 0.5 R HAZ re Hazard Structura nding pote ff, if know ng hazard ing ogic haza	0.3 0.0 NA 0.9 ARDS s That 1 al Evalu ential (un m) s from ta urds or S mage/de	0.9 -0.4 -0.7 0.5	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Ye Detaile Detaile Ye No Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonstru- tailed ev	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Ev. wwn FEM less tha hazards tructural h actural h actural h	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards i azards e is not ne	0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that i cessary	0.8 -0.4 -0.6 0.3 ed? r other bu	0.8 -0.4 -0.7 0.3 wilding	0.6 -0.3 -0.3 0.2 eck one) raluated	0.9 -0.5 NA 1.4
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: Pa Interior: Pa Soil Type Source: Ye Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score, Nonstructural hazards?	rtial s G PERF(SL2] Yes	0.3 0.0 -0.5 1.6 All Sides Visible No	0.6 -0.1 -0.8 1.2 Aer D? N	0.9 -0.3 -1.2 0.8 ial ered	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Pour cut-c Bealin Fallin Geol Geol Sign the s	0.9 -0.5 -0.7 0.5 R HAZ/ R HAZ/ R HAZ/ Structura ding pote ogic hazard ing ogic hazard ifficant dar	0.3 0.0 NA 0.9 ARDS s That 1 al Evalu ential (un m) s from ta urds or S mage/de system	0.9 -0.4 -0.7 0.5 Frigger A ation? Iless SL2 aller adja oil Type terioratio	NA 0.9 -0.5 -0.6 0.5	1.9 0.6 -0.2 -0.6 0.3 ACT Detaile Ye Ve Ve No Detaile	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonstro tailed ev o, no non	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Event own FEM less that hazards tructural h uctural h uctural h uctural h uctural h	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards e is not ne al hazard	0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that cessary is identified	0.8 -0.4 -0.6 0.3 ed? r other bu	0.8 -0.4 -0.7 0.3 iilding ied? (ch uld be ev ire mitig: DNK	0.6 -0.3 -0.3 0.2 eck one) raluated	0.9 -0.5 NA 1.4
Soil Type A or B Soil Type E (1-3 stories) Soil Type E (> 3 stories) Minimum Score, SMIN FINAL LEVEL 1 SCORE, S EXTENT OF REVIEW Exterior: Pa Interior: Pa Interior: Pa Interior: Ve Soil Type Source: Geologic Hazards Source: Contact Person: LEVEL 2 SCREENING Yes, Final Level 2 Score, Nonstructural hazards?	rtial ne s G PERF(SL2	0.3 0.0 -0.5 1.6 Visible No ORME	0.6 -0.1 -0.8 1.2 Aer Entr D? N N e verifie	0.9 -0.3 -1.2 0.8 ial ered o lo	0.6 -0.4 -0.7 0.5 OTHE Are Ther Detailed Pour cut-c Bealin Fallin Geol Geol Sign the s	0.9 -0.5 -0.7 0.5 R HAZA Re Hazard Structura ading pote off, if know ng hazard ing ogic haza ificant dar tructural s	0.3 0.0 NA 0.9 ARDS s That T al Evalu ential (un m) s from ta urds or S mage/de system e follow	0.9 -0.4 -0.7 0.5 Frigger A ation? Iless SL2 aller adja oil Type terioratio	NA 0.9 -0.5 -0.6 0.5 Cent F n to	1.9 0.6 -0.2 -0.6 0.3 Detaile Ye Ve Ve Ve No Detaile Ye No de de No mated o	2.1 0.8 -0.2 -0.8 0.3 ION R ed Struc es, unkno es, score es, other o ed Nons es, nonstructure o, no non <i>r</i> unrelia	NA 0.7 -0.4 -0.4 0.3 EQUIF tural Evi own FEM less tha hazards tructural h actural h aluation structura ble data	2.1 0.9 -0.5 NA 0.3 RED aluation IA buildir n cut-off present I Evalua hazards e is not ne al hazard	0.7 -0.3 -0.5 0.2 Require ng type o tion Rec identified xist that cessary s identified DNK = D	0.8 -0.4 -0.6 0.3 r other bu o not Kr	0.8 -0.4 -0.7 0.3 wilding	0.6 -0.3 -0.3 0.2 eck one) raluated	0.9 -0.5 NA 1.4